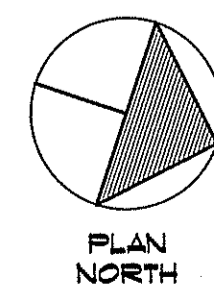
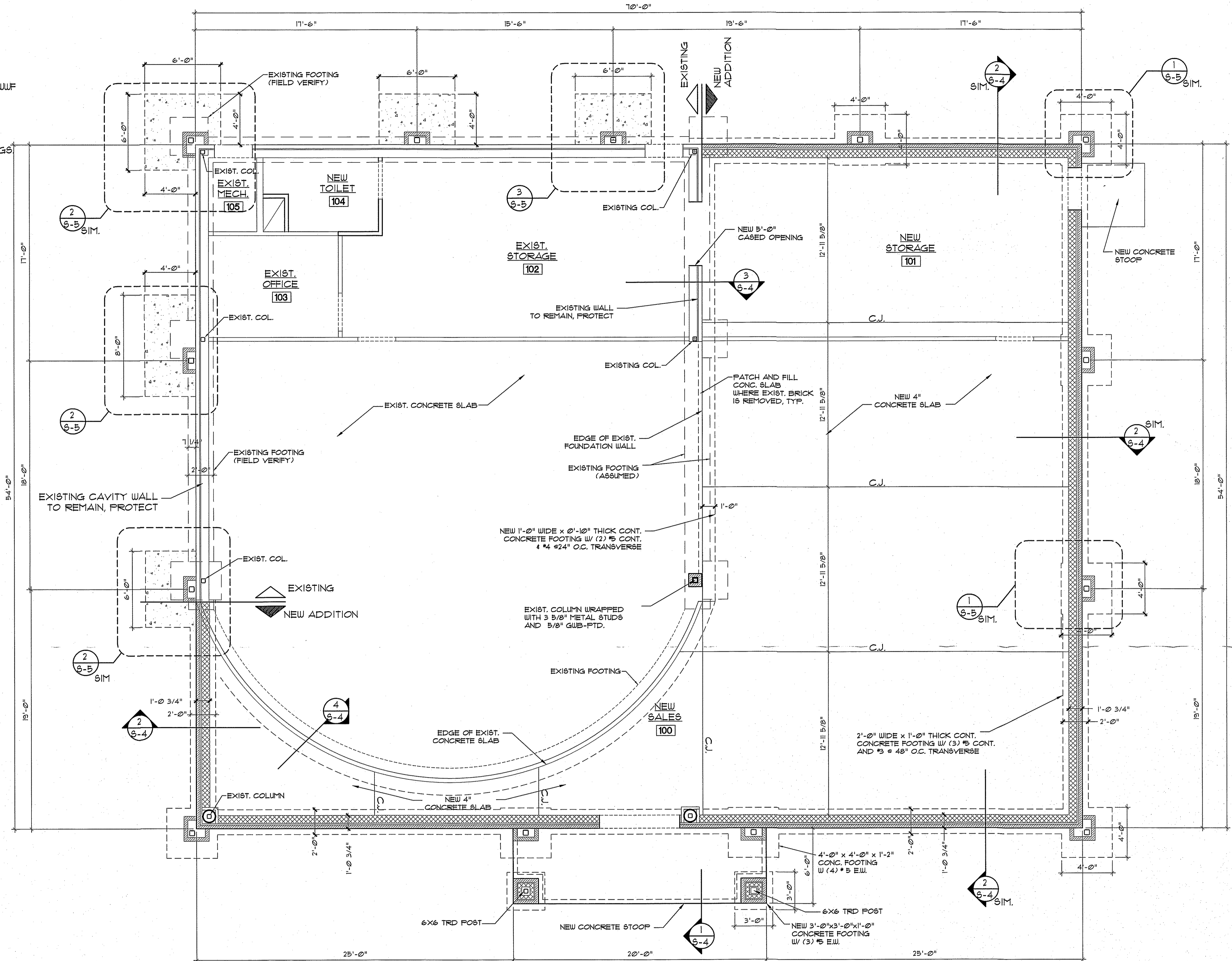
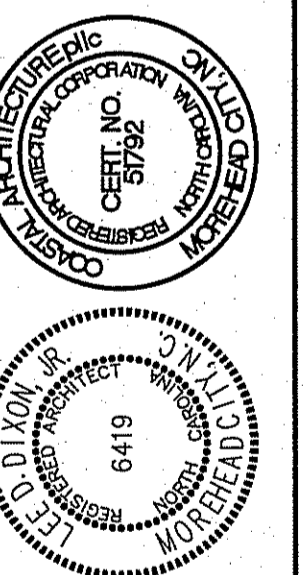
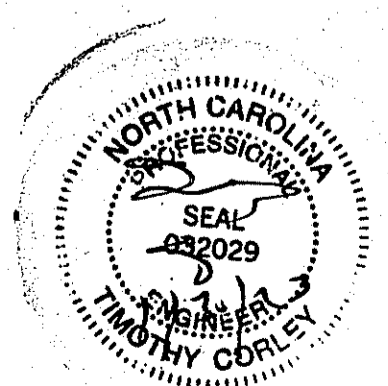


**NOTES:**

1. TOP OF FOOTING = (8" B.F.G.), TYPICAL U.O.N.
2. STEP FOOTING AS REQUIRED.
3. FLOOR SLAB SHALL BE 4" CONCRETE REINF. W/ 6X6 W/4XW/4 W/4F OVER APPROVED VAPOR BARRIER ON 4" MIN. COMPACTED GRANULAR BASE.
4. C.J. DENOTES CONTROL JOINT LOCATION.
5. SEE ARCH PLANS FOR LOCATION OF EXTERIOR DOOR OPENINGS.



**NEW FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"





Barrel Roof Dormer Calculations  
Calculations are rounded to the nearest 1/16 inch

Radius of Barrel Roof = 2'-10''  
Main Roof Pitch = 5.00  
Main Roof Pitch Angle = 22.61986°  
Barrel Roof Plan View Run = 6'-9 5/8''  
Total Arc Length of Barrel Roof = 8'-10 7/8''  
Total Arc Angle of Barrel Roof = 180.00000

Arc Length of 12" O.C. spacing = 1'-0''  
Arc Angle of 12" O.C. Spacing = 20.22204  
Cord Length of 12" O.C. Spacing = 11.93781

Plan View Ellipse Axis  
Semi Minor Axis= 2'-10''  
Semi Major Axis= 6'-9 5/8''

Main Roof Ellipse Axis  
Semi Minor Axis= 2'-10''  
Semi Major Axis= 7'-4 7/16''

Ellipse layout points on Main Roof Real Surface  
Purlin Face cut perpendicular to Roof Surface

#0 [y = 7'-4 7/16'' x = 0] Purlin Slope Angle = 0.00000°  
Miter Angle = 67.38014°  
Saw Blade Bevel Angle = 0.00000°  
Barrel Roof Rafter Length = 6'-9 5/8''

#1 [y = 6'-10 15/16'' x = 0'-11 13/16''] Purlin Slope Angle = 20.22204°  
Miter Angle = 66.05701°  
Saw Blade Bevel Angle = 18.60669°  
Barrel Roof Rafter Length = 6'-4 5/8''

#2 [y = 5'-7 5/16'' x = 1'-10 1/16''] Purlin Slope Angle = 40.44408°  
Miter Angle = 61.29944°  
Saw Blade Bevel Angle = 36.78437°  
Barrel Roof Rafter Length = 5'-2 1/8''

#3 [y = 3'-7 5/16'' x = 2'-5 11/16''] Purlin Slope Angle = 60.66612°  
Miter Angle = 49.61827°  
Saw Blade Bevel Angle = 53.58320°  
Barrel Roof Rafter Length = 3'-3 15/16''

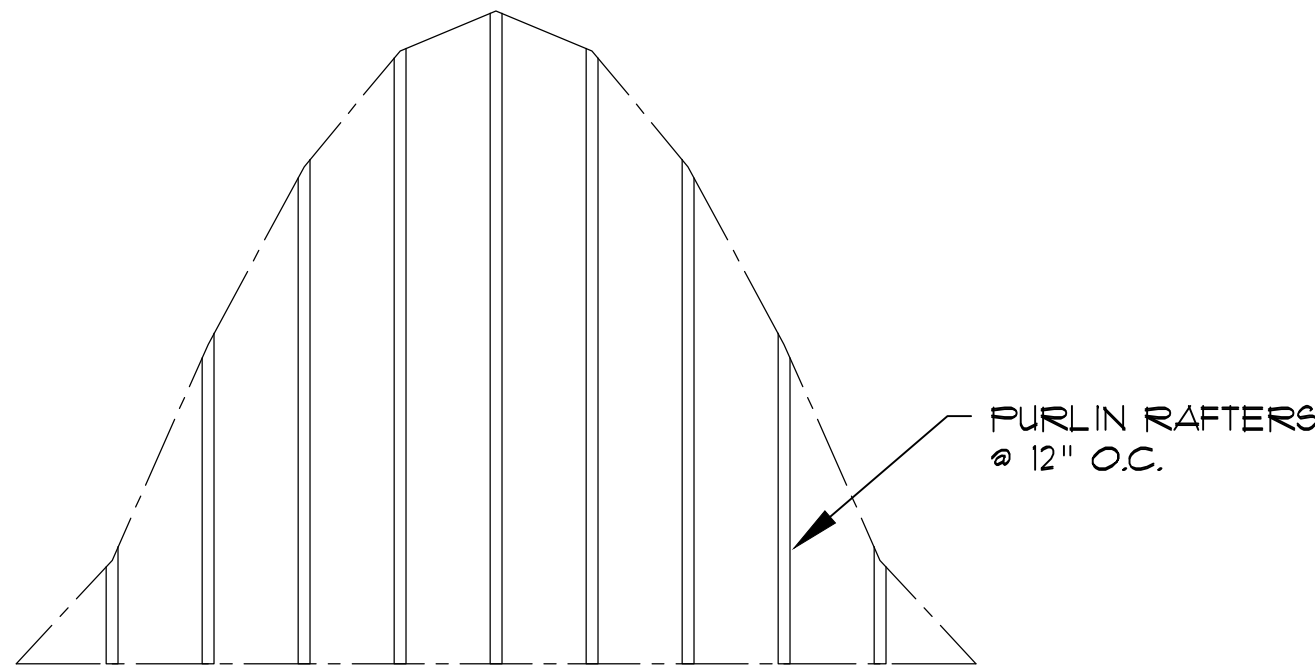
#4 [y = 1'-2'' x = 2'-9 5/8''] Purlin Slope Angle = 80.88816°  
Miter Angle = 20.81025°  
Saw Blade Bevel Angle = 65.70354°  
Barrel Roof Rafter Length = 1'-0 15/16''

#5 [y = 0 x = 2'-10''] Purlin Slope Angle = 90°  
Miter Angle = 0.00000°  
Saw Blade Bevel Angle = 67.38014°  
Barrel Roof Rafter Length = 0

Barrel Roof Dormer Real Roof Surface Layout Points  
(Plywood Roof Sheathing)  
x= 0, y= 6'-9 5/8''  
x= 1'-0'', y= 6'-4 5/8''  
x= 2'-0'', y= 5'-2 1/8''  
x= 3'-0'', y= 3'-3 15/16''  
x= 4'-0'', y= 1'-0 15/16''  
x= 4'-5 7/16'', y= 0

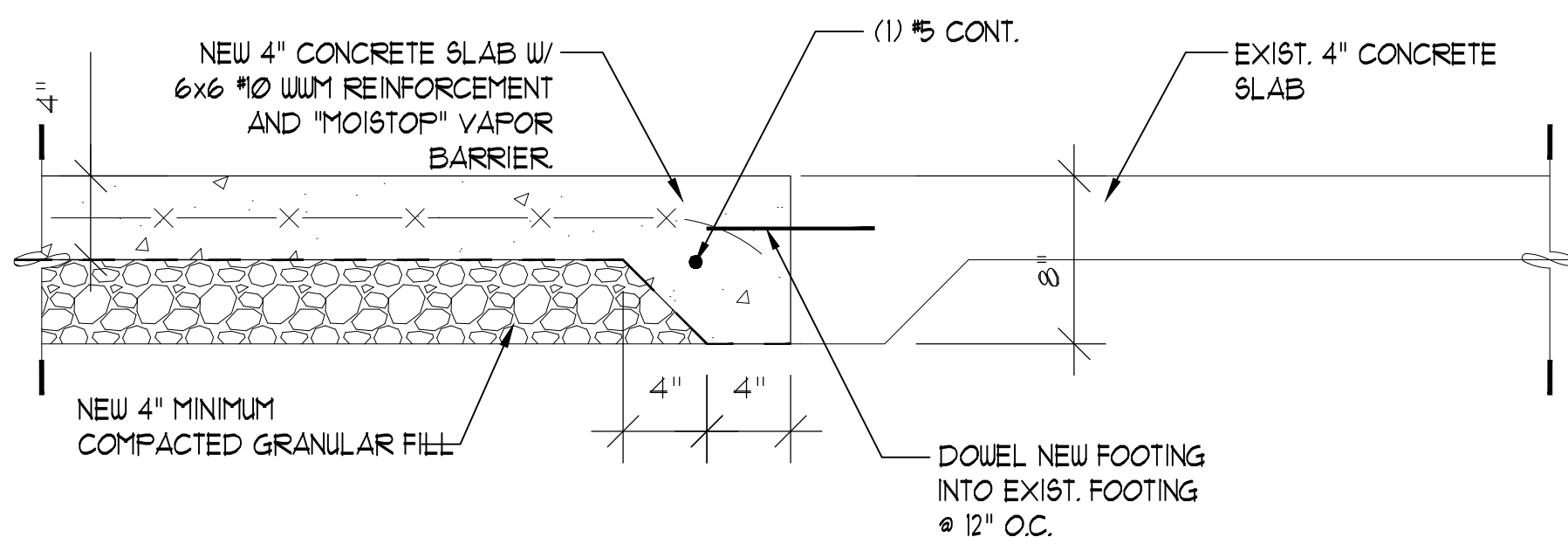
Purlin Rafter Deduction for Ellipse Valley Sleeper Offset for 3/4" Plywood = 0'-1 15/16''  
Purlin Rafter Deduction for Ellipse Valley Sleeper Offset for 1 1/8" Plywood = 0'-2 15/16''  
Purlin Rafter Deduction for Ellipse Valley Sleeper Offset for 1 1/2" Material = 0'-3 15/16''  
Purlin Rafter Deduction for Ellipse Valley Sleeper Offset for 1 3/4" LVL = 0'-4 9/16''

Ellipse Valley Sleeper Offset for 3/4" Plywood = 0'-1 13/16''  
Ellipse Valley Sleeper Offset for 1 1/8" Plywood = 0'-2 3/4''  
Ellipse Valley Sleeper Offset for 1 1/2" Material = 0'-3 5/8''  
Ellipse Valley Sleeper Offset for 1 3/4" LVL = 0'-4 1/4''

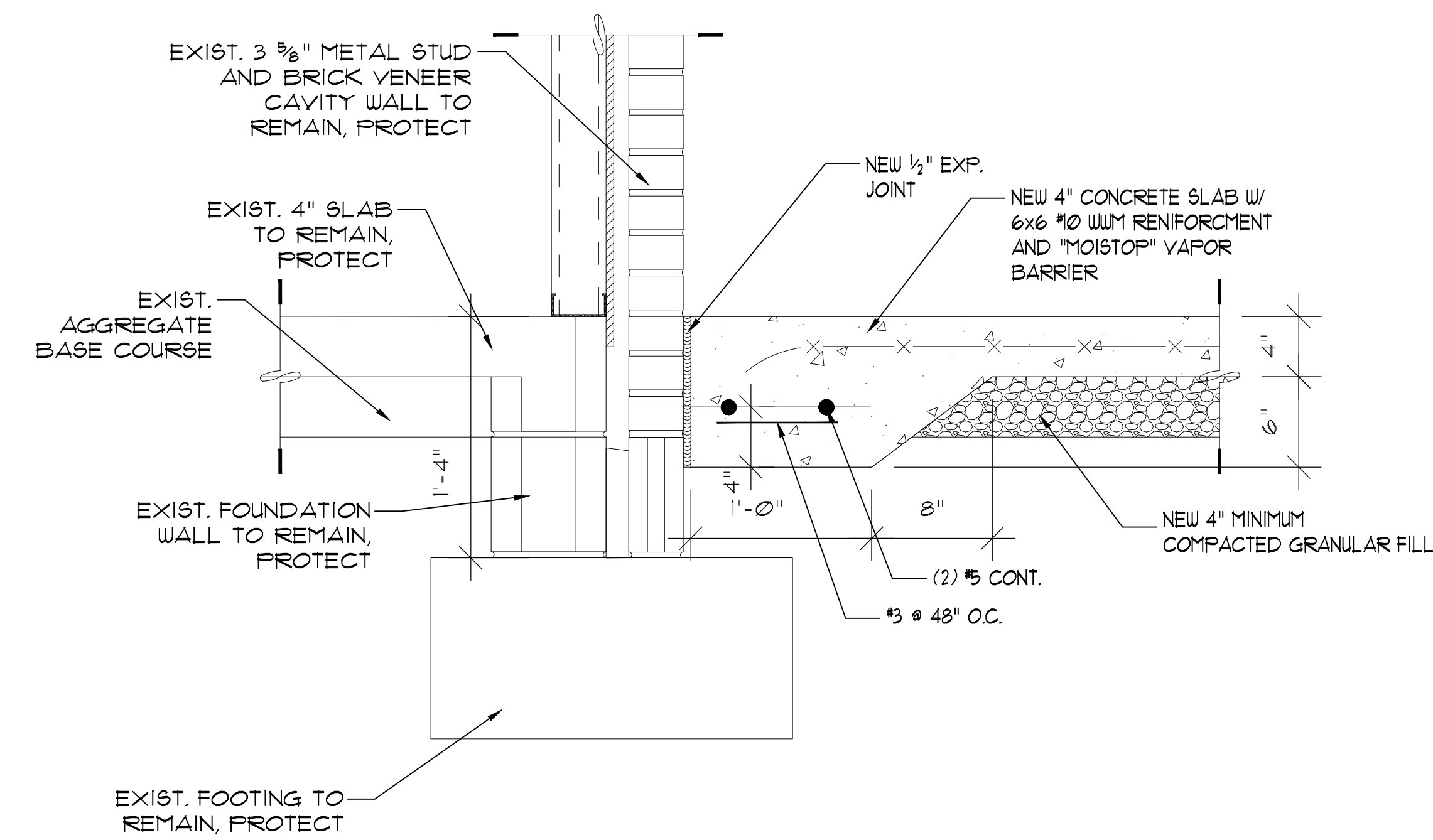


2  
S-3  
BARREL DORMER  
CALCULATIONS  
SCALE: N.T.S.

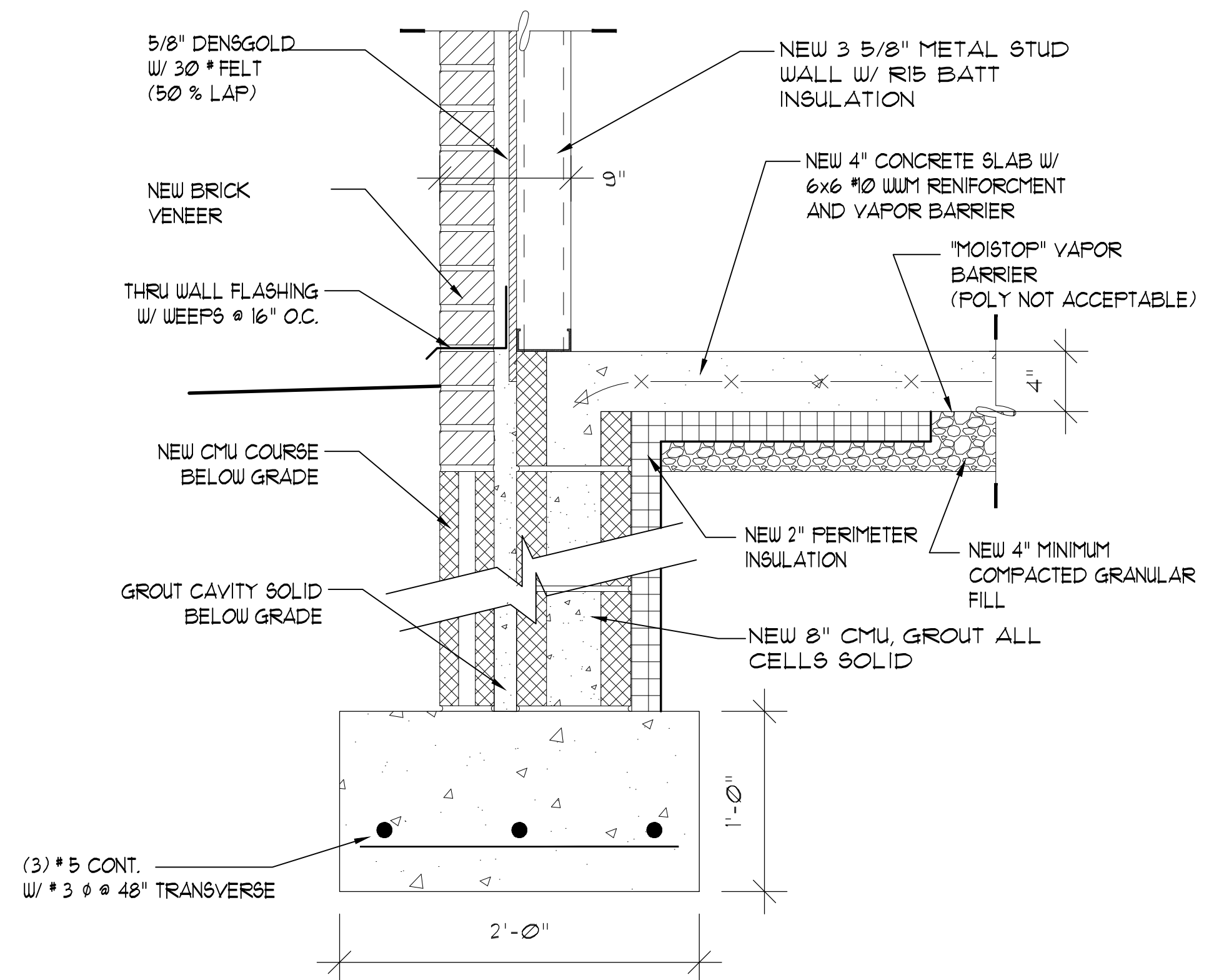
1  
S-3  
BARREL DORMER  
FRAMING PLAN  
SCALE: 1/2"=1'-0"



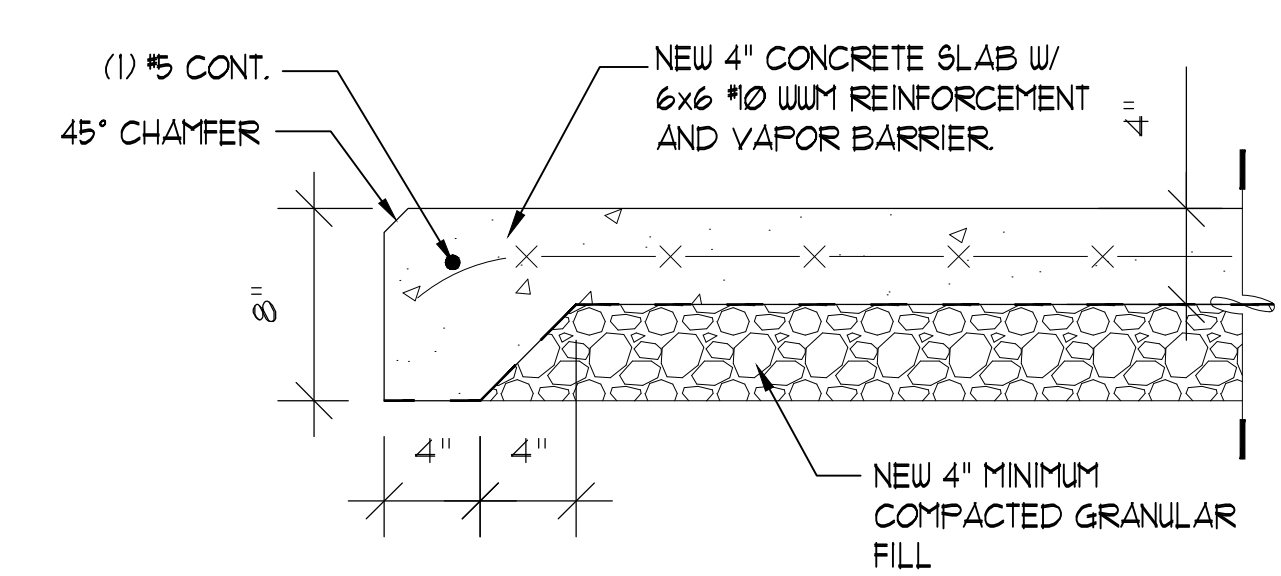
**4**  
S-4  
**SLAB JOINT AT NEW TO EXIST SLAB EDGE DETAIL**  
SCALE: 1 1/2" = 1'-0"



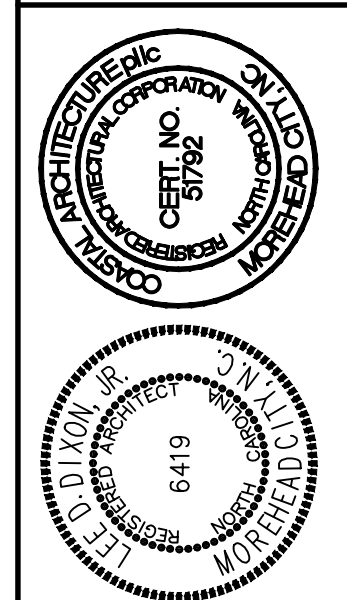
**3**  
S-4  
**NEW SLAB TO EXIST. EXTERIOR WALL DETAIL**  
SCALE: 1 1/2" = 1'-0"



**2**  
S-4  
**NEW WALL FOOTING DETAIL**  
SCALE: 1 1/2" = 1'-0"



**1**  
S-4  
**SLAB EDGE AT NEW STOOP DETAIL**  
SCALE: 1 1/2" = 1'-0"



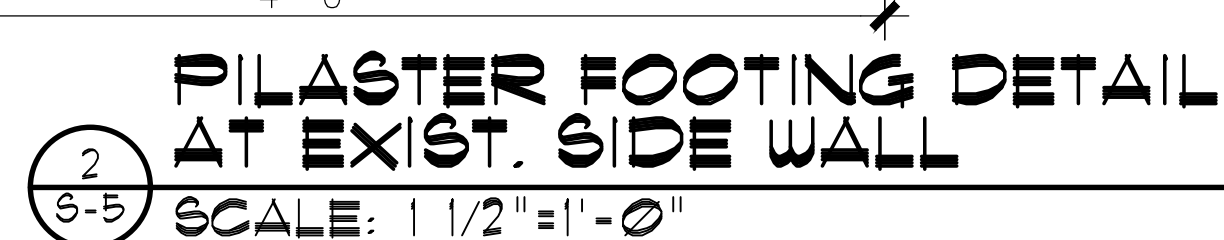
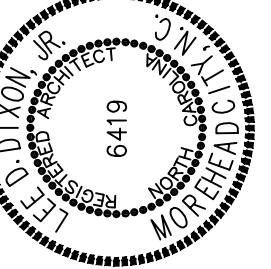
WALL FOOTING DETAILS

**21036**

ISSUED: 01/25/23  
DWG BY: TRA  
CKD BY: LDD

REVISIONS

SHEET NO.  
**S-4**  
OF



GENERAL NOTES:

1. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.
2. ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD AND WITH ALL OTHER DRAWINGS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.
3. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING (AND ACCOMPANYING FOOTINGS), AND GUYS OR TIEDOWNS.
4. ADDITIONAL OBSERVATIONS AS A RESULT OF REJECTION OF WORK COMPLETED AND/OR ADDITIONAL OBSERVATIONS DUE TO THE DISCREPANCIES IN WORK OBSERVED WILL BE AT THE EXPENSE OF THE CONTRACTOR.
5. ALL STRUCTURAL SHOP DRAWINGS TO BE REVIEWED BY JOB SUPERINTENDENT IN ADDITION TO ALL PERSONNEL DEEMED NECESSARY BY CONTRACTOR PRIOR TO SUBMITTAL TO ENGINEER FOR APPROVAL.
6. ALL SHOP DRAWING RESUBMITTALS SHALL INCLUDE A WRITTEN DETAILED LIST OF LOCATIONS AND DESCRIPTIONS OF ALL CHANGES MADE FROM PREVIOUS SUBMITTAL. LIST SHALL BE SPECIFIC AND GENERAL NOTES SUCH AS 'DIMENSIONS CORRECTED' ARE NOT ACCEPTABLE.

DESIGN CODES:

CURRENT NC BUILDING CODE.

ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY.

AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN.

DESIGN CRITERIA:

SEE G-2

COMPONENTS & CLADDING:

ALL BUILDING COMPONENTS AND CLADDING ENGINEERED BY THE COMPONENT MANUFACTURER ARE, TO BE DESIGNED BY THE MANUFACTURER'S ENGINEER FOR WIND LOADS DETERMINED PER THE INTERNATIONAL BUILDING CODE FOR THE BASIC DESIGN WIND VELOCITY, IMPORTANCE FACTOR, AND EXPOSURE LISTED ABOVE.

FOUNDATION NOTES:

1. Foundation design is based on assumed 1,500sf.

2. Spread and continuous footings shall bear a minimum of 24 inches below lowest adjacent finished grade on properly compacted subgrades and select fill with a net allowable bearing capacity of 2,000 PSF.

3. Contractor shall provide for dewatering at excavations from either surface water or seepage.

4. Contractor shall provide adequate shoring to prevent cave-ins.

5. All foundation excavations and subgrade preparation shall be inspected by a representative of the Geotechnical Engineer prior to placement of reinforcing steel or concrete.

PLUMBING SLEEVES:

MINIMUM SLEEVE SPACING SHALL BE TWO DIAMETERS CENTER TO CENTER TO THE LARGER SLEEVE OR 4" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.

CHEMICAL ANCHORS:

SHALL BE A POLYMER INJECTION SYSTEM SUCH AS RAMSET "EPOC", MOLLY "PARAMOUNT HYC", Sika "SKINBUR INJECTION SET", "ULTI-HIGH STRENGTH EPOXY", OR APPROVED EQUAL, INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE.

PENETRATIONS:

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS OTHER THAN THOSE LOCATED ON THESE DRAWINGS WITHOUT PREVIOUS APPROVAL OF THE ENGINEER.

CONCRETE MIX DESIGN:

1. SHALL BE MIX DESIGNED BY A RECOGNIZED TESTING LABORATORY TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX.

3,500 psi – GRADE BEAMS, FOOTINGS, AND SLABS

2. SUBMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAB TESTS FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED. THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1 AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED. OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

3. CONCRETE SHALL UTILIZE TYPE I/II CEMENT UNLESS OTHERWISE DIRECTED BY THE GEOTECHNICAL ENGINEER OR GEOTECHNICAL REPORT.

4. THE CONCRETE STRENGTHS SHOWN IN THE SECTION ABOVE AND IN THE SPECIFICATIONS ARE MINIMUM COMPRESSIVE STRENGTHS. THE ENGINEER SHALL DETERMINE IF THE CONCRETE IS ACCEPTABLE, OR TO BE REMOVED, OR TO RECEIVE SPECIAL CURING IF THE COMPRESSIVE STRENGTHS ARE LESS THAN SPECIFIED.

5. ALL CONCRETE EXPOSED TO WEATHER OR EARTH SHALL BE AIR ENTRAINED 5% TO 7%.

6. WATER REDUCING AGENTS MAY BE USED IN THE CONCRETE MIX. PLASTICIZERS AND SUPER-PLASTICIZERS MAY BE USED ONLY WHEN WRITTEN PERMISSION OF THE ENGINEER IS GIVEN.

7. NO SALTS OF ANY KIND MAY BE USED IN CONCRETE BEFORE OBTAINING THE ENGINEER'S WRITTEN PERMISSION FOR THEIR USE.

8. CONCRETE FOR TROWEL-FINISHED INTERIOR CONCRETE FLOORS SHALL NOT INCLUDE AN AIR-ENTRAINING ADMIXTURE; THE MAXIMUM AIR CONTENT IN THESE SLABS SHALL NOT EXCEED .3%.

CONCRETE AND REINFORCING PLACEMENT:

1. ALL CONCRETE SHALL BE PLACED IN ACCORDANCE WITH ACI 301 AND ACI 117 EXCEPT AS MODIFIED BELOW:

ACI 117 ITEM 4.3.1.1  
ELEVATIONS OF SLABS-ON-GRADE: TOP OF SLAB ELEVATION SHALL BE WITHIN A 3/8" ENVELOPE EITHER SIDE OF THE THEORETICAL DESIGN SURFACE.

ACI 117 ITEM 4.5.7  
FLOOR FINISH TOLERANCES AS MEASURED BY PLACING A FREESTANDING (UNWEIGED) 10 FT. STRAIGHTEDGE ANYWHERE ON THE SLAB AND ALLOWING IT TO REST UPON TWO HIGH SPOTS WITHIN 28 DAYS AFTER ALLOWING IT TO REST UPON TWO HIGH SPOTS WITHIN 28 DAYS AFTER STRAIGHTEDGE AND THE FLOOR SHALL NOT EXCEED 1/4".

2. ALL REINFORCING STEEL TO BE ASTM A615, GRADE 60 (#4 AND LARGER), EXCEPT WHERE NOTED OTHERWISE. REINFORCING SHALL NOT BE WELDED.

3. WELDED WIRE FABRIC TO CONFORM TO ASTM A185 AND SHALL BE FREE FROM OIL, SCALE AND RUST. PLACE WFF IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND THE SPECIFICATIONS. MINIMUM LAPS SHALL BE ONE SPACE PLUS 2".

4. ALL REINFORCING STEEL BARS TO BE DETAILED AND PLACED IN ACCORDANCE WITH THE LATEST ACI MANUALS.

5. LAP ALL REINFORCING SPLICES IN CONCRETE A MINIMUM OF 48 BAR DIAMETERS OR 24 INCHES, WHICHEVER IS GREATER, UNLESS NOTE OTHERWISE ON DRAWINGS (CLASS B SPLICE).

6. PROVIDE CORNER BARS OF SAME BAR DIAMETER AS SPECIFIED FOR THE WALL, BEAM OR FOOTING. PROVIDE MINIMUM OF 40 BAR DIAMETER LAP FOR ALL CORNER BARS, UNLESS NOTED OTHERWISE.

7. PROVIDE FOUNDATION DOWELS AS SHOWN. MINIMUM SIZE DOWELS TO BE #4, UNLESS OTHERWISE NOTED. ALL VERTICAL REINFORCING STEEL IN COLUMNS AND PIERS, OR VERTICAL REINFORCING IN WALLS, SHALL BE TOWELED INTO THE FOOTINGS WITH SAME SIZE AND QUANTITY DOWEL AS THE VERTICAL REINFORCING.

8. WHERE SHOWN ON THE DRAWINGS, PROVIDE WELD PLATES, WELDMENTS, OR CONCRETE INSERTS FOR FASTENING AND SECURING OTHER COMPONENTS. CONCRETE INSERTS SHALL BE FURNISHED BY THE CONTRACTOR. REQUIRING THEM AND INSTALLED BY THE CONTRACTOR CASTING THE CONCRETE AROUND THEM. CLIP ANGLES SHALL BE FURNISHED BY THE CONTRACTOR. REQUIRING THEM.

9. REINFORCING STEEL SHALL RECEIVE CONCRETE COVER AS FOLLOWS:

DESCRIPTIONMINIMUM COVER  
CAST AGAINST AND PERMANENTLY"0"  
EXPOSED TO EARTH

EXPOSED TO EARTH OR WEATHER  
#6 THROUGH #10 BARS"2"  
#5 BARS OR SMALLER1 1/2"

NOT EXPOSED TO EARTH OR WEATHER OR IN CONTACT WITH THE GROUND:  
SLABS AND WALLS  
#11 BARS OR SMALLER3/4"  
#14 AND #161 1/2"

BEAMS AND COLUMNS1 1/2"

10. PROVIDE TWO (2) #6'S, ONE AT EACH FACE, UNLESS NOTED OTHERWISE, AROUND ALL OPENINGS NOT GREATER THAN 12"x12" IN CAST-IN-PLACE CONCRETE. EXTEND REINFORCING 2'-4" BEYOND OPENING IN BOTH DIRECTIONS. CONTACT ENGINEER FOR ALL OPENINGS GREATER THAN 12"x12" FOR DESIGN.

11. COLD WEATHER AND HOT WEATHER PROVISIONS OF ACI 306 AND 305 (CURRENT EDITIONS), RESPECTIVELY, SHALL BE MAINTAINED.  
12. CONTRACTOR TO FURNISH AND INSTALL 500 LINEAR FT. EACH OF ADDITIONAL #4 & #5 REINFORCING STEEL TO BE USED AT ENGINEER'S DISCRETION.

FORMWORK AND SHORING:

NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO-THIRDS OF THE 28 DAY DESIGN STRENGTH. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESOURCES SHALL MEET THE REQUIREMENTS SET FORTH IN ACI STANDARDS 301 AND 347.

ANCHOR RODS:

1. The building manufacturer shall be responsible for anchor rod size (i.e., diameter), spacing, and location.

2. All anchor rods shall be fabricated in accordance with ASTM F1554 grade 36, class 2A.

3. Anchor rod nuts shall conform to ASTM A563, heavy hex, grade A. All washers shall conform to ASTM F436, type I.

3. Anchor rods shall have a plain finish.

STRUCTURAL STEEL:

1. STEEL SHALL CONFORM TO ASTM A992 (Fy=50 ksi) FOR ALL W-SHAPES, AND ASTM A36 (Fy=36 ksi) FOR ALL OTHER MISCELLANEOUS SHAPES AND PLATES. STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B (Fy=46 ksi). STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, GRADE B, TYPE "D" OR "S" (Fy=42 ksi).

2. STEEL SHALL CONFORM TO THE LATEST EDITION OF "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC).

3. ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR SHALL BE HOT-DIPPED GALVANIZED.

4. ALL SHOP CONNECTIONS TO BE WELDED (UTILIZING E70XX ELECTRODES) AND FIELD CONNECTIONS TO BE BOLTED, UNLESS OTHERWISE NOTED. STEEL TO RECEIVE ONE SHOP COAT AND ONE FIELD TOUCH UP COAT OF APPROVED PAINT, EXCEPT WHERE GALVANIZED IS INDICATED ON THE DRAWINGS.

5. WELDS FOR ALL EXPOSED STRUCTURAL STEEL SHALL BE GROUND SMOOTH, UNLESS NOTED OTHERWISE.

6. ALL BOLTED CONNECTIONS SHALL CONSIST OF 3/4" DIAMETER (MIN.) ASTM A325 HIGH STRENGTH BOLTS, UNLESS NOTED OTHERWISE.

MASONRY NOTES:

1. All concrete block units shall conform to ASTM C 90.

2. All concrete block above grade shall be light weight with a minimum ultimate compressive strength (F'm) of 1500 PSI.

3. Mortar shall conform with ASTM C 270, Type S, with a minimum compressive strength of 1800 PSI @ 28 days.

4. Grout shall be classified as fine grout and have a slump of nine (9) inches and a minimum compressive strength of 2500 PSI and shall conform to ASTM C 476. All blocks with vertical reinforcing shall be grouted full height. All bond beams shall be grouted full. The maximum grout pour height shall not exceed five (5) feet.

5. All top of walls shall have bond beams with two (2) #4's continuous unless noted otherwise. Install #4 corner bars at all bondbeam intersections and corners.

6. All concrete block wall intersections and corners shall be built integrally of use "3012" wall anchors by Duro-D-Wall (or equivalent) @ 16" o.c. vertical spacing.

## TIMBER FRAMING NOTES:

1. Sawn timber members shall conform to the requirements of the American Institute of Timber Construction Standards, AITC 117 & A190.1, latest edition.

2. Sawn lumber shall be #2 K.D. spruce-pine-fir (SPF), moisture content 15 percent with the following allowable stresses for a 2x4:

F(b) = 975 psi, F(t) = 450 psi, F(c)(parallel to grain) = 1,150 psi  
F(c)(perpendicular to grain) = 425 psi, F(v) = 135 psi,  
E = 1,400,000 psi

3. N/A

4. All roof trusses (24" o.c.) shall be designed by the fabricator to support 10 PSF dead load on the top chord and 5 PSF dead load on the bottom chord plus 20 psf live load on the top chord, plus all other code loads which apply to the specific geographic region.

5. N/A

6. Truss fabricator shall be responsible for engineering all connections to load bearing walls.

7. Contractor shall provide additional bridging and bracing for truss chords and web members beyond what is shown as required by the truss fabricator's design, net uplift, and temporary bracing for during erection. The floor and roof systems are not stable until sheathing and permanent bracing is in place.

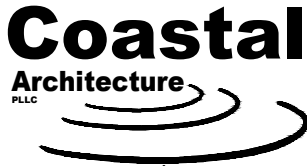
8. The truss fabricator shall submit calculations with shop drawings stamped by a professional engineer registered in the State of North Carolina to the Architect for review prior to fabrication of trusses. The shop drawings must include truss bracing drawings for both the roof and floor trusses.

9. Truss manufacturer shall design and provide truss hangers where trusses are supported by other trusses.

10. Plywood for roof decking shall conform to the requirements in DOC PS 1 or PS 2. Each panel shall be identified for grade and glue type by the trademarks of an approved testing and grading agency. Roof decking shall be 5/8 inch, 5-ply, C-D INT-APA with exterior glue, Exposure 1 (span index 48/24). Plyclips shall be used at all free edges, one at mid-point between all supports. Roof Zone 1/2/3 nailing for plywood roof sheathing shall be 8d nails at 4/4/6 inches on center along plywood edges respectively and 12 inches on center along intermediate members (Ref. ASCE 7-10 Components and Cladding).

11. All sawn lumber members in contact with concrete or masonry shall be pressure treated with a water borne treatment to a net retention of 0.3 PCF.

12. Fasten 3/4" Advantech floor decking to trusses with 10d nails or equivalent at 6 inches on center at panel edges and diaphragm boundaries and at 12 inches on center along intermediate members.



- Architectural Design
- Planning
- Interiors



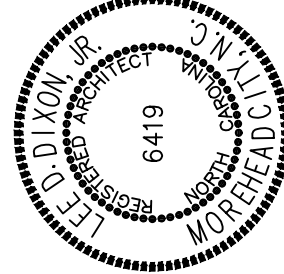
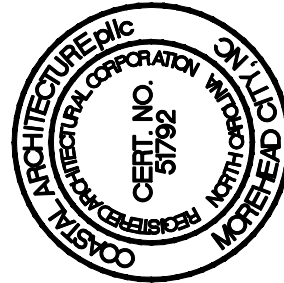
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ADDITION TO  
ABC NEWPORT  
NEWPORT, NORTH CAROLINA



STRUCTURAL NOTES

21036

ISSUED: 01/25/23

DWG BY: MES

CKD BY: LDD

REVISIONS


SHEET NO.

S-6  
OF