



Eddy Building – Detached Addition : Addendum #3

DATE: October 16, 2024

Hobbs Architects, PA
159 West Salisbury Street
PO Box 1457
Pittsboro, North Carolina 27312

ADDENDUM #3:

TO: All bidders of record, which have received plans and specifications for the project referred to above.

This Addendum No. 3 forms a part of the Contract Documents and modifies the original Drawings and Specifications dated September 6, 2024, as noted below. Acknowledge receipt of this addendum in the space provided on the Bid Form. Failure to do so may subject bidder to disqualification.

This addendum consists of 12 pages of information including: six (6) 8.5x11 sheets and six (6) 24x36 sheets.

General:

- A. **The Bid Date has been revised to be October 30 at 1:30 pm at the front desk of the Donald W. Eddy Building, 4300 Reedy Creek Rd, Raleigh, NC 27607**
- B. Bidders are reminded that the Pre-Bid Conference was mandatory for General Contractors.
- C. All email correspondence should be sent to the following:
 - Chevon Moore, Hobbs Architects, at cmoore@hobbsarchitects.com
 - Krista Millard, Hobbs Architects, at kmillard@hobbsarchitects.com

Clarifications/Questions and Answers:

1. Question: Suggest an allowance for permit fees, tap fees, facility fees, etc. Looking at the City of Raleigh fee schedule, some of these fees are very high for 3" water service. In addition, since it's a State of NC project, wouldn't the fees be waived.
 1. RESPONSE: Fees will not be waived by the City of Raleigh. GC's are to include in their bids all required permit fees, tap fees, and facility fees.
2. I have a few subcontractors that would like to visit the site. What is the protocol for that?
 1. RESPONSE: A site visit date and time will be arranged in the coming week and will be announced via Planscope to all plan holders and pre-bid attendees.
3. C-100 note #6 states that not all vegetation to be demolished as shown. Please clarify.
 1. RESPONSE: It is incumbent on the contractor to visit the site and include the necessary demolition of existing vegetation in the base bid. No alterations to the contract will be accepted for this.
4. C-100 concrete to be demolished. Is this contract work or part of add alternate G1A?

1. RESPONSE: This scope is part of add alternate G1A.
5. Is there impact resistant sheetrock on all 4 walls? Detail 2/A-501 shows it and detail 1/A-501 does not.
 1. RESPONSE: Interior drywall has been removed from the project – see Addendum #2.
6. Is framing required to be installed behind the impact resistant sheetrock or secured directly to the steel wall girts as shown?
 1. RESPONSE: Interior drywall has been removed from the project – see Addendum #2.
7. The asphalt paving that is existing to remain is in very poor condition. Should bidders submit unit pricing to replace the existing asphalt?
 1. RESPONSE: Yes, Sections 01 21 00 Allowances and 01 22 00 Unit Prices will be included in a future addendum. The Bid Form will also be reissued.
8. Who is the AHJ that will issue the permit and what allowance should the GC's carry for permit cost?
 1. RESPONSE: The City of Raleigh is the Authority Having Jurisdiction on the project. GC's will be responsible for all site permitting costs (zoning, storm, etc.). The design team will provide the approved drawings for the contractor's use to coordinate and pull the required permits. There is no building permit required for the project.
9. Can you please clarify if the aluminum cantilevered canopy alternate over the dock expansion will have to be supported by the new storage building or not ? If so, would the additional cost to support the canopy be part of Alternate pricing?
 1. RESPONSE: The "PRE-FABRICATED ALUMINUM CANTILEVERED LOADING DOCK CANOPY" is a stand-alone element and is independent of any other building in the project.
10. The drawings say that the rod-hung canopies would be by PEMB manufacturers. Most of the PEMB manufacturers do not provide that, can you please let us of any other approved manufacturers for the same?
 1. RESPONSE:
 1. It is acceptable to provide entrance/overhead-supported canopies by a manufacturer other than the PEMB manufacturer, however, the PEMB manufacturer would be responsible for coordinating the canopy as to load and connection.
 2. If otherwise in compliance with the contract documents, acceptable rod-hung entrance/overhead-supported canopy manufacturers include the following:
 1. Peachtree Protective Covers, www.peachtreecovers.com, Lithia Springs, GA, (800)341-3325
 2. Mitchell Metals, www.mitchellmetals.net, Smyrna, GA, (770)766-5521
 3. SKYSCAPE Architectural Canopies, www.skyscapecanopies.com, Duluth, GA, (700)674-5452
 4. MASA Architectural Canopies, www.architecturalcanopies.com, Piscataway, NJ, (732)453-6120

3. Alternative manufacturers will be considered if shown by GC to adhere to the contract documents.
11. The specs have a soffit panel listed, but we don't see any mention of soffit panels on the drawings. Are there any areas that will have soffit panels on this building?
 1. RESPONSE: No, soffit panels shall be removed from the project.
12. Are all columns at the finish floor? Or will they be above or below FF? Looks like the column at line 1F may be 4'-0" AFF but I am not sure. Please verify.
 1. RESPONSE:
 1. The PEMB columns install at the top of slab on grade (-4'-0") per plans.
 2. The Pre-fabricated Aluminum Cantilevered loading dock canopy columns (part of Alternate G-1B) install at the top of the loading dock slab (the slab is part of Alternate G-1A).
 3. At column line 1F, there is a grated steel platform, stair, and pit for the lift, however the top of slab elevation is maintained for the PEMB columns.
13. Drawings are shown with Insulated Metal panels on both roof and walls – also says see specifications. Once we review the specs, they mention standard single-skin panels with blanket insulation. Which is correct – Drawings or Specifications?
 1. RESPONSE: Insulated Metal Panels are to be used for both roof and walls per the drawings. A specification for these panels shall be issued in a forthcoming addendum.
14. Can you please confirm if it would be required for us to provide a site construction trailer on site during the entire duration of the project?
 1. RESPONSE: Contractor is to provide a site construction trailer at a minimum for the period of time necessary to get the addition dried-in. At that point, the GC may choose to have the construction office inside the addition.
15. Is framing to be 2"x4-1/2" thermal storefront, clear anodized?
 1. RESPONSE: Yes. See also Specification Section 08 41 13 Aluminum-Framed Entrances and Storefronts for more information.
16. Is door to be medium-stile with 10-inch bottom-rail
 1. RESPONSE: Yes, see door type 'D' on Sheet A-901.
17. Any low-e coating, tint, film, etc on the glass units?
 1. RESPONSE: Yes, all exterior glass in windows and doors to be Glass Type G-1 as defined in Specification Section 08 80 00 Glazing, Part 3.7. Glass to be tempered where indicated in drawings and required by code.
18. Wall section 2/A501 show drywall to the full building height. Detail 1/A501 does not show any drywall. Please confirm which walls are to receive the drywall.

1. RESPONSE: This was answered in Addendum #2. All interior drywall was removed from the project.
19. There is a key note 2 on S112 but we are not seeing it on the plans. Please clarify.
1. RESPONSE: Keynote is clarified on revised structural plans included in this addendum. Keynote 02 refers to the loading dock slab. See revised drawings.
20. The concrete slab is 1 ¼ » larger than the grid lines of the building. It appears this is for a building with standard metal wall panels. Since we are using insulated metal panels, should that dimension be increased?
1. RESPONSE: This has been corrected on the revised structural drawings included in this addendum. The slab edge is to align with the gridlines around the perimeter of the building as shown in detail 3/A-501.
21. What R value is required for the IMP panels?
1. RESPONSE:
 1. Per Specification Section 13 34 19 Metal Building Systems, Part 2.3.M, Roof R-value is to be minimum R-30 and Wall R-value is to be minimum R-20.
22. What is the basis for design or approved manufacturers for the Prefabricated Aluminum Cantilevered Dock Canopy?
1. RESPONSE:
 1. The basis of design for the Prefabricated Aluminum Cantilevered Dock Canopy is Peachtree Protective Covers, www.peachtreecovers.com, Lithia Springs, GA, (800)341-3325
 2. Other acceptable manufacturers, if in compliance with contract documents, include the following:
 1. Mitchell Metals, www.mitchellmetals.net, Smyrna, GA, (770)766-5521
 2. Tennessee Valley Metals, www.tvmetals.com, Oneonta, AL (205)274-9500
23. We need a detail and design for the steel grated platform that extend the loading dock to the edge of the scissor lift. The scissor lifts are independent and do not have a platform. Can you provide us with this design for the steel platform and the steps from the edge of the new loading dock to the scissor lift?
1. RESPONSE: The steel grated platform and associated stairs are delegated design. See Spec Sections 05 51 19 Metal Grating Stairs and 05 52 13 Pipe and Tube Railings included in the Project Manual.
24. Ok – the platform is a design-build. What are the design criteria? Platform - dead load. Size of the grate openings.
1. RESPONSE: The steel grated platform and associated stairs are delegated design. See Spec Sections 05 51 19 Metal Grating Stairs and 05 52 13 Pipe and Tube Railings included in the Project Manual.

25. Unit Prices? In the SCO sample Contract page 3 o4 you list Unit Prices, but we cannot find a list of unit prices. Are there Unit Prices for this Project?

1. RESPONSE: Section 01 22 00 Unit Prices will be issued in a future addendum.

26. Are design calculations required for storefronts? Openings are small and wind loads are low for the area.

1. RESPONSE: Storefronts to be per Specification Section 08 41 13 Aluminum Framed Entrances and Storefronts.

27. Can the existing building be shut down for approx. 3 hours for the domestic and fire to connect?

1. RESPONSE: This would be challenging for the Owner to shut down the building during regular working hours. Though the Owner will work with the awarded GC to the extent possible, it is likely that the building would need to be shut down during off hours, the weekend, or a state holiday.

28. Canopy Sub RFI: Would it be acceptable to have the single post supported cantilevered canopy shown on detail 1/A-502 to have a support bracket in the corner on a 45-degree angle? My subcontractors are saying that there would have to be one there to support the canopy as currently drawn due to cantilever.

1. RESPONSE: A 45-degree angle is not acceptable if it conflicts with adjacent construction or projects into minimum 7'-0" head clearance. Otherwise, a 45-degree angle is acceptable, but not preferred.

29. GC RFI: Can you confirm all testing is to be by owner?

1. RESPONSE: All construction materials testing will be by Owner.

30. Our PEMB manufacturer has asked for an extension on the bid date as they do not feel they would have enough time to bid on the project with the answers to the questions releasing in the Addendum on 10/16. Can you please review this request for an extension on the bid date giving them enough time to review the responses to the RFI.

1. RESPONSE: The bid date has been moved to October 30 at 1:30 pm at the front desk of the Donald W. Eaddy Building, 4300 Reedy Creek Rd, Raleigh, NC 27607.

31. Can you confirm no allowances are listed in specs?

1. RESPONSE: Section 01 21 00 Allowances will be issued in a future addendum.

32. The Carrier 48LC units that are specified for this project are no longer in production. Please provide a replacement model.

1. RESPONSE: Contractors are to provide a unit with equivalent performance that uses R454B

33. Are there any requirements for a BDA system or testing?

1. RESPONSE: No, BDA systems are not required for buildings under 7,500 SF

34. "What panels are we to use on the roof/walls? The spec calls out single skin SSR 24 gauge and 26 gauge PBR panels. The Architectural drawings call out Insulated metal panels. Can you let us know which route we should go with these?"

1. RESPONSE: Insulated Metal Panels are to be used for both roof and walls per the drawings. A specification for these panels shall be issued in a forthcoming addendum.

END OF ADDENDUM #3

GENERAL NOTES:

- THE STRUCTURAL DRAWINGS MUST BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS, AND THE SPECIFICATIONS. THE CONTRACTOR MUST VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES, AND ADDITIONAL ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF THE NORTH CAROLINA STATE BUILDING CODE, 2018 EDITION.
- THE CONTRACTOR MUST PROVIDE TEMPORARY SHORING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL PERMANENT SUPPORTS AND LATERAL BRACING ARE IN PLACE.
- PORTIONS OF THE EXISTING STRUCTURE NOT ALTERED AND NOT AFFECTED BY THE ALTERATION HAVE NOT BEEN REVIEWED FOR COMPLIANCE WITH THE CODE REQUIREMENTS FOR A NEW STRUCTURE.
- BEFORE PROCEEDING WITH WORK WITHIN THE EXISTING STRUCTURE, THE CONTRACTOR MUST BECOME FAMILIAR WITH THE EXISTING STRUCTURAL CONDITIONS. ANY SHORING OR BRACING SHOWN IS A PARTIAL AND SCHEMATIC REPRESENTATION OF THAT REQUIRED. THE CONTRACTOR MUST BE SOLELY RESPONSIBLE FOR THE DESIGN AND ERECTION OF ANY AND ALL SAFEGUARDS NECESSARY TO PROTECT THE EXISTING STRUCTURE. THE CONTRACTOR MUST PROVIDE SHORING, BRACING, AND OTHER SAFEGUARDS TO MAINTAIN ALL PARTS OF THE STRUCTURE IN A SAFE CONDITION AT ALL TIMES DURING THE PROCESS OF DEMOLITION AND CONSTRUCTION.
- DISCREPANCIES BETWEEN DRAWINGS, BETWEEN THE DRAWINGS AND THE SPECIFICATIONS, OR WITHIN THE SPECIFICATIONS, MUST BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER DURING THE BIDDING PROCESS IN TIME TO PERMIT CLARIFICATION BY ADDENDUM. IF INCONSISTENCIES, DISCREPANCIES OR CONTRADICTIONS IN THE CONTRACT DOCUMENTS ARE DISCOVERED AFTER THE CLOSE OF BIDDING QUESTIONS, THE CONTRACTOR MUST BE DEEMED BY SUBMITTAL OF THEIR BID, TO HAVE BID THE MOST COSTLY AS TO LABOR, MATERIALS, DURATION, SEQUENCE AND METHOD OF CONSTRUCTION TO PROVIDE THE WORK.
- PRIOR TO ISSUING THE STRUCTURAL DRAWINGS FOR ANY PURPOSE, AUTHORIZATION MUST BE OBTAINED FROM THE STRUCTURAL ENGINEER OF RECORD. WHEN AUTHORIZED, THE DOCUMENTS THAT ARE RELEASED MUST BE CLEARLY IDENTIFIED WITH THE AUTHORIZED PURPOSE AND MUST INCLUDE THE DATE OF RELEASE.

8. DESIGN CRITERIA:

LIVE LOADS - CONCENTRATED

FLOOR	2,000#
STORAGE FLOOR	3,000#

LIVE LOADS - UNIFORM:

SLAB ON GRADE/ PLATFORM	125 PSF
ROOF	20 PSF

UNLESS OTHERWISE NOTED, CONCENTRATED LOADS ARE UNIFORMLY OVER 2'-6" x 2'-6 AREA.

SNOW LOADS:

GROUND SNOW LOAD	15 PSF
FLAT ROOF LOAD	15 PSF
IMPORTANCE FACTOR (Is)	1.0
THERMAL FACTOR (Ct)	1.0
EXPOSURE FACTOR (Ce)	1.0

WIND LOADS:

ULTIMATE DESIGN WIND SPEED (VULT)	115 MPH
NOMINAL DESIGN (VASD) WIND SPEED	90 MPH
EXPOSURE CATEGORY	B
INTERNAL PRESSURE COEFFICIENT	±0.18

COMPONENT AND CLADDING PRESSURES:

WALLS, ZONE 4 (>50 SF)	15 PSF
WALLS, ZONE 5 (50 SF)	20 PSF
ROOF, ZONE 1	24 PSF
ROOF, ZONE 2	40 PSF
ROOF, ZONE 3	55 PSF

WIND BASE SHEAR:

N-S	.16K
E-W	.17K

SEISMIC LOADS:

SITE CLASSIFICATION	D		
SEISMIC DESIGN CATEGORY	B		
IMPORTANCE FACTOR (IE)	1.0		
SPECTRAL RESPONSE ACCELERATIONS:			
Ss	0.154	S1	0.077
Sms	0.247	Sm1	0.184
Sds	0.165	Sd1	0.123

RAIN LOADS:

RAIN INTENSITY (15 MIN)	6.5 IN/HR
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SEISMIC BASE SHEAR (ASSUMED PROPERTIES):

N-S	.7K
E-W	.7K

FOUNDATION NOTES:

- FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING REPORT PREPARED BY FROEHLING & ROBERTSON, DATED FEBRUARY 9, 2023.
- FOUNDATIONS HAVE BEEN DESIGNED FOR A NET ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF.
- PRIOR TO PLACING FOUNDATION CONCRETE, ALL FOUNDATION EXCAVATIONS MUST BE INSPECTED BY THE OWNER'S GEOTECHNICAL TESTING AGENCY TO EXPLORE THE EXTENT OF LOOSE, SOFT, EXPANSIVE, OR OTHERWISE UNSATISFACTORY SOIL MATERIAL AND TO VERIFY DESIGN BEARING PRESSURE. DIRECTION FOR CORRECTIVE ACTION WILL BE PROVIDED BY THE OWNER'S GEOTECHNICAL TESTING AGENCY WHERE UNSATISFACTORY SOILS ARE PRESENT.
- CONTROL GROUNDWATER AND SURFACE RUNOFF THROUGHOUT THE CONSTRUCTION PROCESS. INUNDATION AND LONG TERM EXPOSURE OF BEARING SURFACES WHICH RESULT IN DETERIORATION OF BEARING MUST BE PREVENTED.
- RETAINING WALL HAVE BEEN DESIGNED FOR THE FOLLOWING LATERAL LOAD CRITERIA:

AT-REST PRESSURE (RESIDUAL)	64 PCF
ACTIVE SOIL PRESSURE (GW)	30 PCF
SOIL DENSITY	120 PCF
SOIL COEFFICIENT OF FRICTION	0.35 (ASSUMED)
ALLOWABLE BEARING PRESSURE	2,000 PSF

CAST-IN-PLACE CONCRETE NOTES:

- CONCRETE MUST BE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301 AND 318.
 - CONCRETE MUST BE NORMAL WEIGHT AND MUST OBTAIN 28 DAY COMPRESSIVE STRENGTHS AS FOLLOWS:
- | | |
|---------------------------------|-----------|
| A. SLAB-ON-GRADE | 4,000 PSI |
| B. CONCRETE NOT OTHERWISE NOTED | 3,000 PSI |
| C. FOUNDATIONS | 3,000 PSI |
- REINFORCING MATERIALS MUST BE AS FOLLOWS:
- | |
|--|
| A. REINFORCING BARS - ASTM A615, GRADE 60, DEFORMED. |
| B. WELDED REINFORCING BARS - ASTM A706, GRADE 60. |
| C. WELDED WIRE REINFORCEMENT - ASTM A1064, WELDED STEEL WIRE REINFORCEMENT; PROVIDE SHEET TYPE, ROLL TYPE IS NOT ACCEPTABLE. |
- ALL REINFORCING STEEL AND EMBEDDED ITEMS SUCH AS ANCHOR RODS AND WELD PLATES MUST BE ACCURATELY PLACED AND ADEQUATELY TIED AND SUPPORTED BEFORE CONCRETE IS PLACED TO PREVENT DISPLACEMENT BEYOND PERMITTED TOLERANCES.
 - CONCRETE COVER TO REINFORCING STEEL MUST CONFORM TO THE MINIMUM COVER RECOMMENDATIONS IN ACI 318, UNLESS THE DRAWINGS SHOW GREATER COVER REQUIREMENTS.
 - LAP CONTINUOUS REINFORCING STEEL 57 X BAR DIAMETER, TYPICAL UNLESS OTHERWISE NOTED.

ABBREVIATIONS:

ARCH	ARCHITECT	EOS	EDGE OF SLAB	MATL	MATERIAL	SOG	SLAB-ON-GRADE
BLDG	BUILDING	EQ	EQUAL	MAX	MAXIMUM	STD	STANDARD
BOD	BOTTOM OF DECK	EW	EACH WAY	MECH	MECHANICAL	T&B	TOP & BOTTOM
BOT, B	BOTTOM	EXIST	EXISTING	MFR	MANUFACTURER	TOC	TOP OF CONCRETE
BRG	BEARING	EXP	EXPANSION	MID	MIDDLE	TOF	TOP OF FOOTING
BTWN	BETWEEN	EXT	EXTERIOR	MIN	MINIMUM	TOS	TOP OF STEEL
CJ	CONTROL JOINT	FDN	FOUNDATION	MOD	MODIFY	TS	THICKENED SLAB
CL	CENTERLINE	FF EL	FINISHED FLOOR ELEVATION	NTS	NOT TO SCALE	TYP	TYPICAL
CLR	CLEAR	FRMG	FRAMING	OC	ON CENTER	UON	UNLESS OTHERWISE NOTED
COL	COLUMN	OPNG	OPENING	PCY	POUNDS PER CUBIC YARD	VERT	VERTICAL
CONC	CONCRETE	PCY	POUNDS PER CUBIC YARD	PEMB	PRE-ENGINEERED METAL BUILDING	W/	WITH
CONN	CONNECTION	PL	PLATE			WWR	WELDED WIRE REINFORCING
CONSTR	CONSTRUCTION	REF	REFERENCE, REFER TO				
CONT	CONTINUOUS	REINF	REINFORCE, REINFORCED, REINFORCING				
COORD	COORDINATE	REQD	REQUIRED				
CTR	CENTER	REQMTS	REQUIREMENTS				
DIA, Ø	DIAMETER	SCHED	SCHEDULE				
DWGS	DRAWINGS	SF	STEPPED FOOTING				
EA	EACH	SIM	SIMILAR				
EJ	EXPANSION JOINT	SJ	SAWED JOINT				
EL	ELEVATION						
ELEV	ELEVATOR						
EMBED	EMBEDMENT						

METAL BUILDING SYSTEM NOTES:

- METAL BUILDING SYSTEM MUST BE IN ACCORDANCE WITH THE METAL BUILDING MANUFACTURERS ASSOCIATION (MBMA) "DESIGN PRACTICES MANUAL."
 - SUBMIT SHOP DRAWINGS SIGNED AND SEALED BY A NORTH CAROLINA LICENSED PROFESSIONAL ENGINEER RESPONSIBLE FOR THE DESIGN OF METAL BUILDING SYSTEMS. SHOP DRAWINGS MUST INCLUDE DESIGN LOADINGS AND REACTIONS APPLIED TO THE SUPPORTING STRUCTURE. INCLUDE A SUMMARY OF CONTROLLING LOAD CASE FOR EACH LOCATION.
 - METAL BUILDING SYSTEMS MUST BE DESIGNED FOR THE LOAD INDICATED IN THE GENERAL NOTES AND AS FOLLOWS:
- | | |
|---------------------|---|
| A. DEAD LOADS | WEIGHT OF ALL SUPPORTED EQUIPMENT, PLUS WEIGHT OF THE BUILDING. |
| B. COLLATERAL LOADS | 5 PSF |
- THE CONTRACTOR MUST BE RESPONSIBLE FOR THE COORDINATION AND COSTS ASSOCIATED WITH A CONTRACTOR INITIATED CHANGE IN BUILDING MODEL OR MANUFACTURER, INCLUDING CONSTRUCTION COSTS AND RE-ENGINEERING COSTS.
 - THE DESIGN REACTIONS USED ARE INDICATED IN THE SCHEDULE AND PROVIDED BY CECO BUILDING SYSTEMS DATED MAY 3, 2023 AT GRIDLINE 1 OF THE PEMB (SOUTH ELEVATION) THE BID ALTERNATE INCLUDES BRICK CLADDING. LIMIT THE MAXIMUM OUT OF PLANE DEFLECTION OF THE PEMB TO L/240 FOR THE COMBINATION OF FRAME AND GIRT DEFLECTION FOR THIS BID ALTERNATE. LIMIT THE IN-PLANE DEFLECTION OF THE SOUTH WALL TO L/600."

NOTE: LOADS SHOWN BELOW ARE SERVICE LOADS, ASD.

COLUMN GRID	GRAVITY (KIPS)	UPLIFT (KIPS)	SHEAR (KIPS)
E-4	6.6	2.7	0.9
C-4	8.3	2.7	1
B-4	6.6	2.9	0.9
F-4	2.2	1.5	1.4
A-4	2.2	1.5	1.4
F-3	23.4	7.2	19.6
A-3	25	7.4	20.9
F-2	23.4	7.2	19.6
A-2	25	7.4	20.9
F-1	1.0	1.2	0
E-1	5.5	6.5	2.0
C-1	3.3	5.0	2.0
B-1	5.5	7.8	2.6
A-1	1.0	1.0	0

- FOUNDATIONS HAVE BEEN DESIGNED USING THE REACTIONS PROVIDED BY CECO BUILDING SYSTEMS. ANY INCREASE IN REACTION LOAD WILL RENDER THE DESIGNS HEREIN NULL AND VOID.

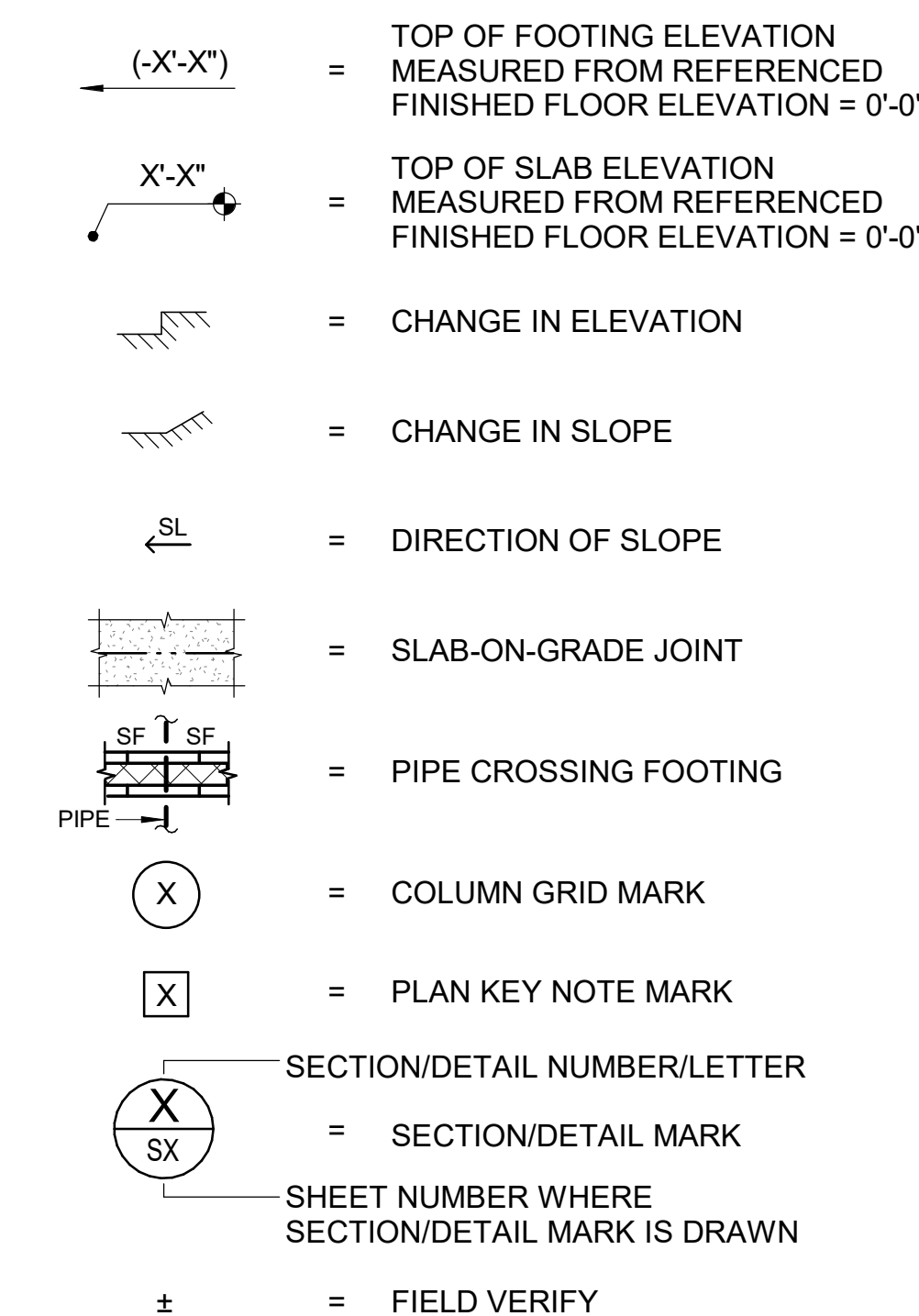
SPECIALTY STRUCTURAL ELEMENTS:

- THE FOLLOWING BUILDING ELEMENTS REQUIRE DELEGATED DESIGN AND ENGINEERING BY A SPECIALTY STRUCTURAL ENGINEER REGISTERED IN THE STATE OF NORTH CAROLINA:
- | |
|---|
| A. METAL STAIRS, HANDRAILS AND PRE-ENGINEERED PLATFORMS |
| B. CURTAIN WALL AND GLAZING ASSEMBLIES INCLUDING CONNECTIONS TO THE STRUCTURE |
| C. COLD-FORMED METAL FRAMING (CFMF) |
| D. PRE-FABRICATED CANOPIES AND AWNINGS |
| E. PRE-ENGINEERED METAL BUILDINGS |
- SUBMIT COMPLETE CALCULATIONS AND SHOP DRAWINGS, SIGNED AND SEALED BY THE PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF NORTH CAROLINA RESPONSIBLE FOR THE DESIGN, INCLUDING DESIGN LOADINGS AND REACTIONS APPLIED TO THE SUPPORTING STRUCTURE. INCLUDE A SUMMARY OF THE CONTROLLING LOAD CASES FOR EACH LOCATION.
 - IN ADDITION TO THEIR OWN DEAD WEIGHT AND THE DEAD LOADS SHOWN OR INDICATED IN THE DRAWINGS, MEMBERS MUST BE DESIGNED TO SUPPORT THE LOADS INDICATED IN THE GENERAL NOTES.
 - THE CONTRACTOR MUST BE RESPONSIBLE FOR THE COORDINATION OF ALL SPECIALTY STRUCTURAL ELEMENTS AND COST ASSOCIATED WITH A CONTRACTOR INITIATED CHANGE IN BUILDING STRUCTURE, INCLUDING CONSTRUCTION COSTS AND RE-ENGINEERING COSTS.

POST-INSTALLED ANCHOR NOTES:

- ALL POST INSTALLED ANCHORS INDICATED ON THE DRAWINGS ARE BY HILTI, INC, AND MUST BE CONSIDERED THE BASIS OF DESIGN PRODUCT. WHERE NOT EXPLICITLY INDICATED IN THE DRAWINGS, THE FOLLOWING ANCHORS/ADHESIVES MUST BE USED:
- | |
|---|
| A. ANCHORAGE TO CONCRETE |
| 1. ADHESIVE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE: |
| a. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) AND VC 20/40 VACUUM SYSTEM (VC 20-U OR VC40U) WITH STEEL THREADED ROD PER ICC ESR-3187. |
| b. ALTERNATIVE ANCHORS: AT-XP BY SIMPSON (INCREASED EMBEDMENT DEPTH NEEDED), OR AC208+ BY DEWALT |
| 2. SCREW ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE: |
| a. HILTI KWIK HUS EZ SCREW ANCHORS PER ICC ESR-3027. |
| b. ALTERNATIVE ANCHORS TITEN HD BY SIMPSON OR SCREW BOLT+ BY DEWALT |
| B. REBAR DOWELING INTO CONCRETE |
| 1. ADHESIVE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE: |
| a. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) AND VC 20/40 VACUUM SYSTEM (VC 20-U OR VC 40-U) WITH CONTINUOUSLY DEFORMED REBAR PER ICC ESR-3187. |
| b. ALTERNATIVE ANCHORS: AT-XP BY SIMPSON (INCREASED EMBEDMENT DEPTH NEEDED), OR AC208+ BY DEWALT |
- ALTERNATE POST INSTALLED ANCHOR PRODUCTS MAY BE SUBMITTED TO THE ENGINEER FOR REVIEW AND POSSIBLE APPROVAL. ALL SUBSTITUTION REQUESTS MUST BE ACCOMPANIED BY AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE. ALTERNATE PRODUCTS MAY REQUIRE MODIFICATIONS TO ANCHOR DIAMETER, SPACING, AND EMBEDMENT.
 - INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.
 - THE CONTRACTOR MUST ARRANGE FOR AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF ANCHOR INSTALLATION.
 - ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
 - EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR MUST LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY FERROSCAN OR GPR.
 - ALL POST INSTALLED ANCHORS REQUIRE CONTINUOUS SPECIAL INSPECTIONS TO VERIFY INSTALLATION HAS BEEN PERFORMED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. REFERENCE THE STATEMENT AND SCHEDULE OF SPECIAL INSPECTIONS FOR ADDITIONAL INFORMATION.

PLAN LEGEND:



THESE DRAWINGS ARE RELEASED FOR THE FOLLOWING USE, AND OTHER USE OF THESE DRAWINGS IS AT THE RISK OF THE CONTRACTOR OR OTHERS USING THESE DRAWINGS FOR THAT UNAUTHORIZED USE. LYNCHMYKINS IS NOT RESPONSIBLE FOR ADDITIONAL COSTS DUE TO CHANGES, COORDINATION OR ADDITIONAL SCOPE OF WORK REQUIRED DUE TO SUCH UNAUTHORIZED USE.

- PRELIMINARY BUDGET PRICING
- EARLY FOUNDATION PACKAGE
- MILL ORDER PACKAGE
- EARLY STEEL PACKAGE
- PERMIT SET
- CONSTRUCTION SET

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NORTH CAROLINA
PROFESSIONAL SEAL
1025948
JAMES S. BATHKATE
2024.09.08

NEW PROJECT FOR
NC DEPT. OF AGRICULTURE & CONSUMER SCIENCES
SCO ID#: 22-24471-02A
EADDY BUILDING DETACHED ADDITION
4300 REEDY CREEK RD., RALEIGH, NC 27607

No.	Date
2	10/16/2024

GENERAL NOTES

DATE
09-06-24

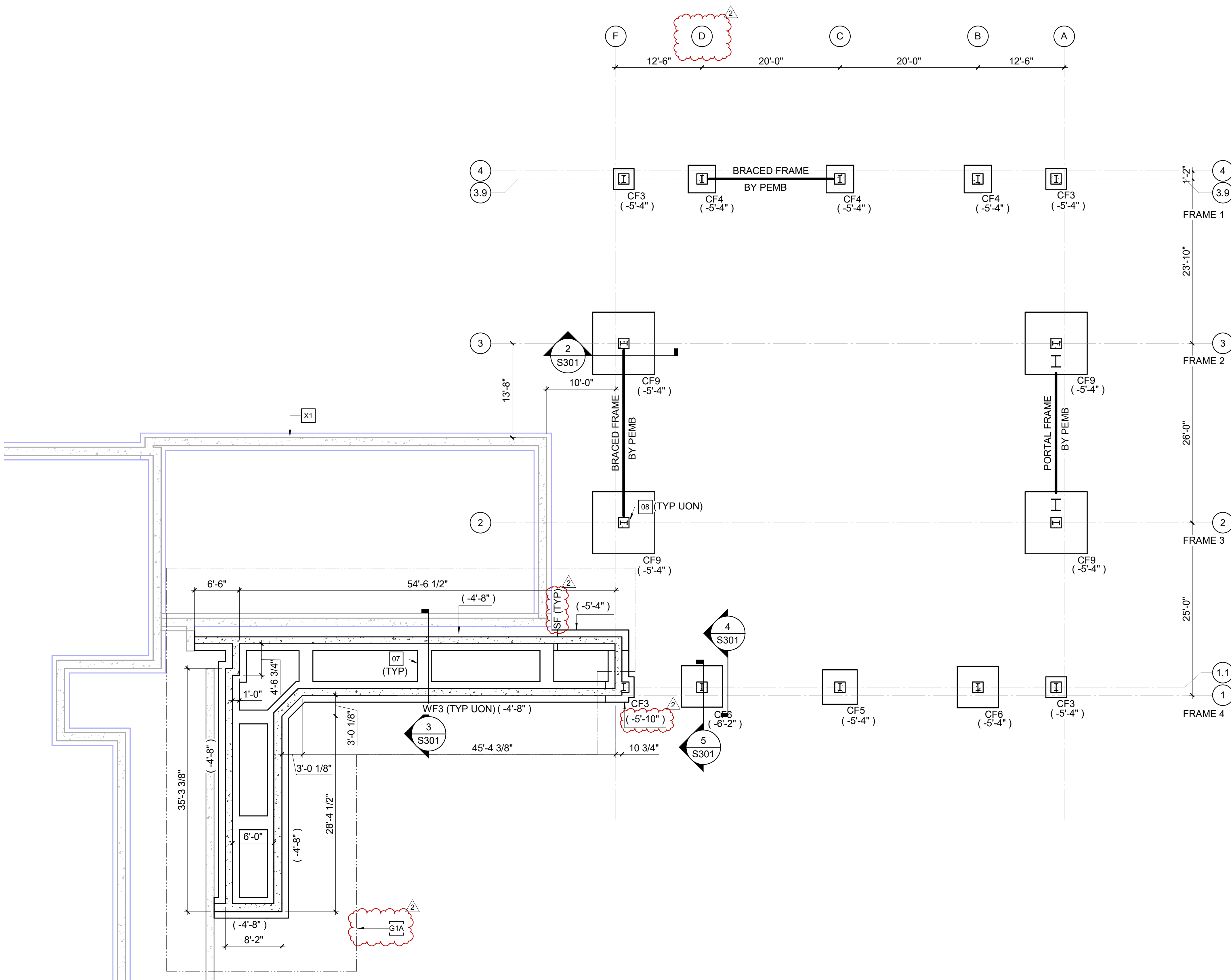
S001

WALL FOOTING SCHEDULE				
MARK	SIZE		REINFORCING	
	WIDTH	DEPTH	CONTINUOUS	TRANSVERSE
WF3	3' - 0"	1'-0"	(4) #4 BOT	#4 AT 24" OC BOT

COLUMN FOOTING SCHEDULE					
MARK	SIZE			REINFORCING	
	LENGTH	WIDTH	DEPTH	BOTTOM	TOP
CF3	3' - 0"	3' - 0"	1'-6"	(4) #5 EW	(4) #5 EW
CF4	4' - 0"	4' - 0"	1'-6"	(5) #5 EW	(5) #5 EW
CF5	5' - 0"	5' - 0"	1'-6"	(6) #5 EW	(5) #5 EW
CF6	6' - 0"	6' - 0"	1'-6"	(7) #5 EW	(7) #5 EW
CF9	9' - 0"	9' - 0"	1'-6"	(10) #5 EW	(10) #5 EW

FOUNDATION PLAN NOTES

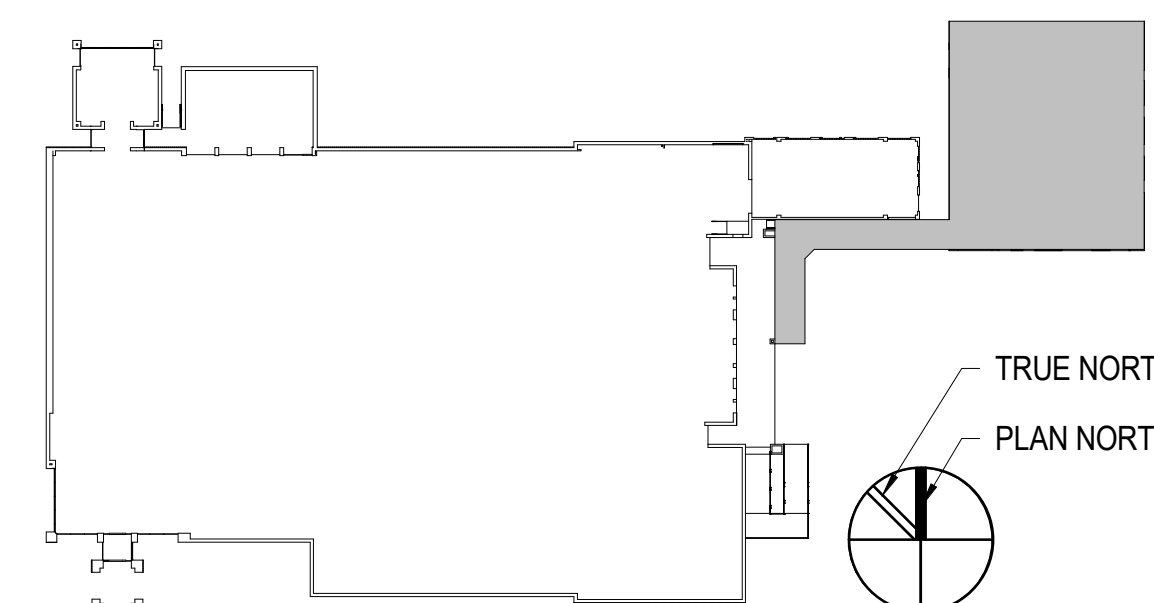
- A. REFERENCE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO NONBEARING WALLS, WALL CONTROL JOINTS AND OPENINGS.
- B. UNLESS OTHERWISE NOTED, ALL ELEVATIONS ARE BASED ON A FINISHED FIRST FLOOR REFERENCE OF 0'-0". ACTUAL FINISHED FLOOR ELEVATION IS 447.5'. REFERENCE ARCHITECTURAL DRAWINGS FOR FINISHED FLOOR MATERIALS.
- C. TOP OF ALL FOOTINGS MUST BE AT ELEVATION -4'-8" UNLESS OTHERWISE NOTED.
- D. NOT ALL UTILITY LOCATIONS ARE SHOWN ON PLAN. THE CONTRACTOR MUST COORDINATE THE LOCATIONS, SIZES, AND INVERTS OF UTILITIES. AT LOCATIONS WHERE UTILITIES PASS BELOW THE TOP OF FOOTING ELEVATION, STEP THE TOP OF FOOTING DOWN ON EACH SIDE PER THE "STEPPED FOOTING DETAIL" AND SLEEVE THE UTILITY THROUGH THE FOUNDATION WALL. THE CONTRACTOR MAY, AT HIS/HER OPTION, SLEEVE THE UTILITY THROUGH THE FOUNDATION PER THE "UTILITY SLEEVE DETAIL".
- E. UNLESS OTHERWISE INDICATED, EXTEND WALL FOOTINGS A MINIMUM OF 6 INCHES BEYOND ENDS OF WALLS.
- F. WHERE PIPE AND UTILITIES CROSS PERIMETER FOUNDATIONS, REFER TO "TYPICAL PIPE PENETRATION THROUGH PERIMETER FOOTING DETAILS." PIPES AND UTILITIES SHALL NOT RUN UNDERNEATH COLUMN FOUNDATIONS.



KEY NOTES

- 07 24" WIDE x 12" DEEP GRADE BEAM WITH (4) #5 CONTINUOUS REINFORCING TOP AND BOTTOM.
- 08 18" CONCRETE PEDESTAL
- G1A ALTERNATE G1A: REFERENCE ARCHITECTURAL DRAWINGS FOR BID/ALTERNATE DESCRIPTION.
- X1 EXISTING WALL.

KEY PLAN



1 FOUNDATION PLAN
1/8" = 1'-0"

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FOUNDATION PLAN

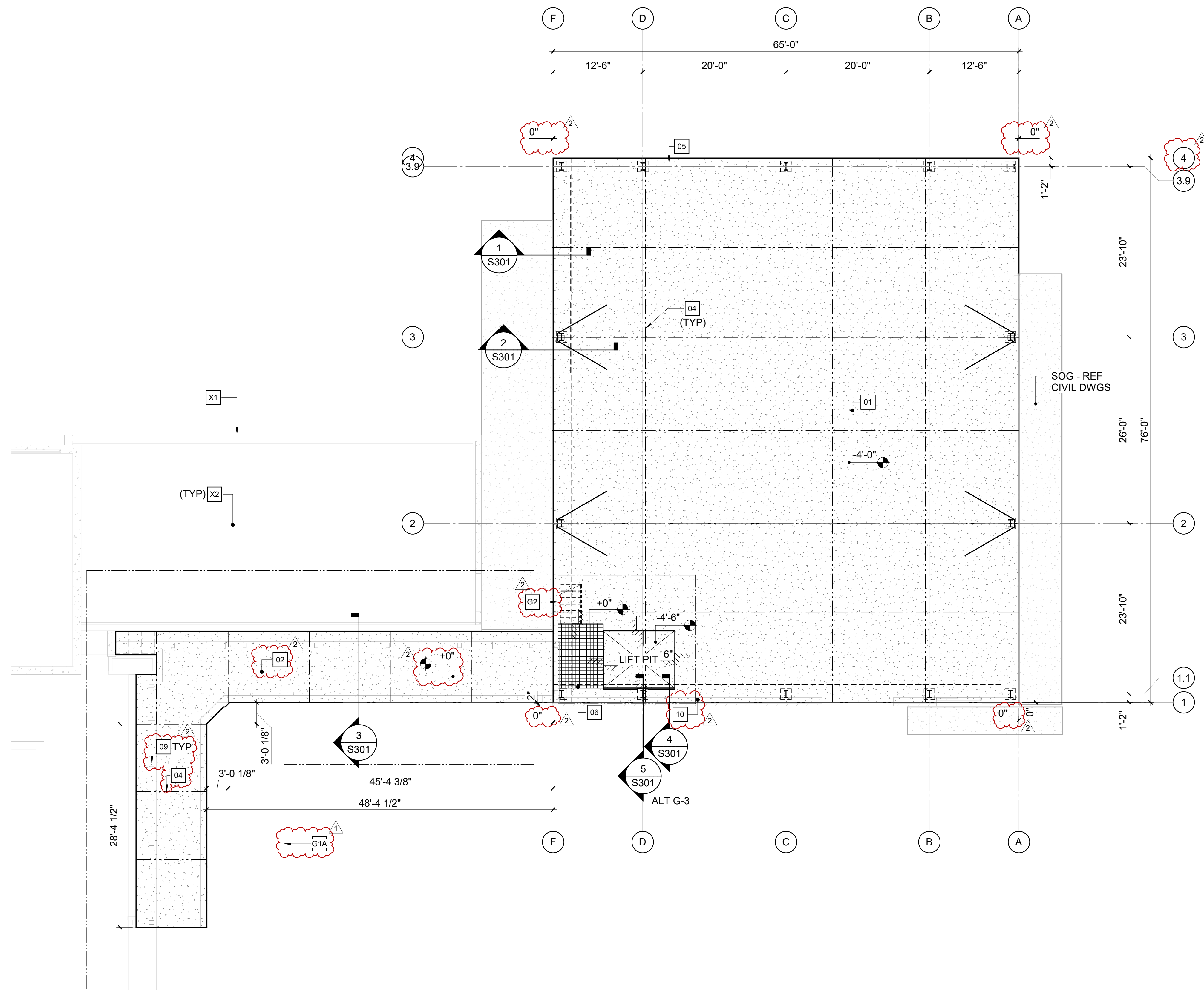
DATE
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1 SLAB PLAN
1/8" = 1'-0"



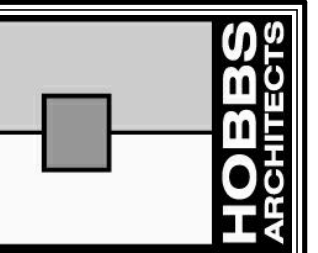
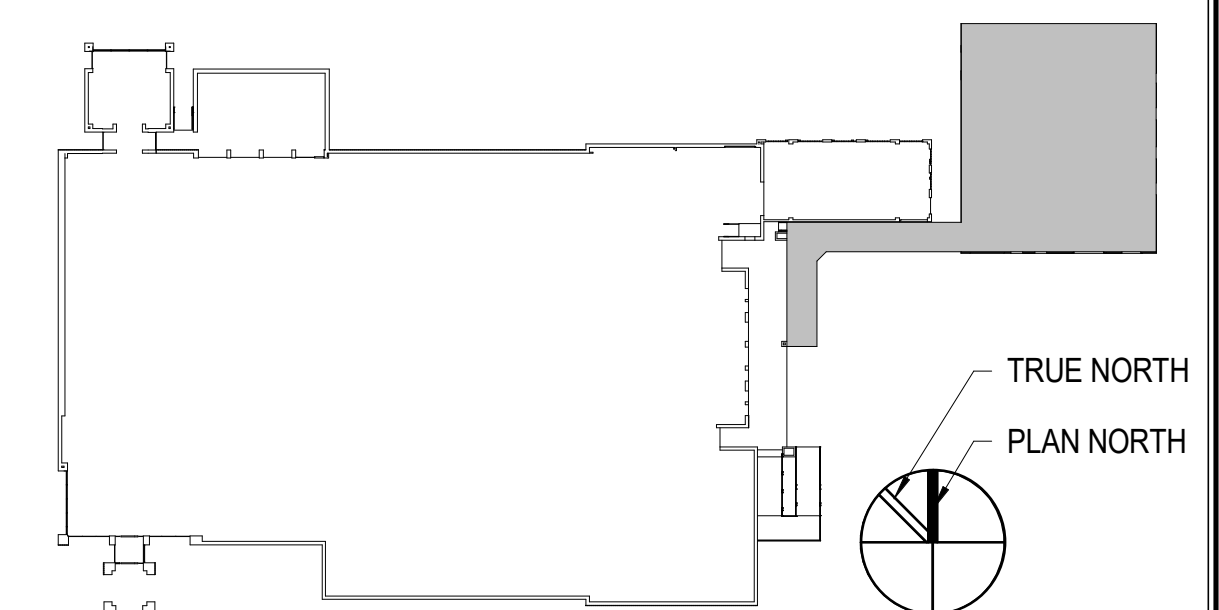
SLAB PLAN NOTES

- A. REFERENCE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO NONBEARING WALLS, WALL CONTROL JOINTS AND OPENINGS.
- B. UNLESS OTHERWISE NOTED, ALL ELEVATIONS ARE BASED ON A FINISHED FIRST FLOOR REFERENCE OF 0'-0". ACTUAL FINISHED FLOOR ELEVATION IS 447.5'. REFERENCE ARCHITECTURAL DRAWINGS FOR FINISHED FLOOR MATERIALS.
- C. REFERENCE ARCHITECTURAL DRAWINGS FOR EXACT LIMITS OF SLAB DEPRESSIONS AND OMITTED SLABS.
- D. DIMENSIONS SHOWN ON FOUNDATION PLAN ARE TO COLUMN GRIDLINES AND OUTSIDE FACE OF FOUNDATION WALLS, UNLESS OTHERWISE NOTED
- E. FLOOR SINKS AND DRAINS ARE NOT SHOWN ON PLAN. REFERENCE PME DRAWINGS FOR LOCATIONS.
- F. REFERENCE CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE SLABS AND PAVING.
- G. SLAB-ON-GRADE JOINTS MUST BE SAWS JOINTS OR FORMED CONSTRUCTION JOINTS, UNLESS OTHERWISE NOTED. CONTRACTOR MUST COORDINATE ALL SLAB JOINTS WITH JOINTS IN BONDED FLOOR FINISHES. REFERENCE ARCHITECTURAL DRAWINGS FOR FLOOR FINISH JOINT LOCATIONS.
- H. PLACE (1) #4 x 3'-0" IN MIDDLE OF SLAB AT RE-ENTRANT CORNERS WHERE A SLAB JOINT DOES NOT OCCUR.

KEY NOTES

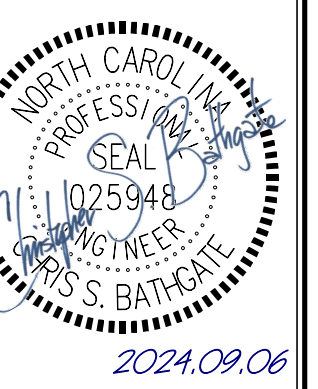
- 01 6" CONCRETE SLAB-ON-GRADE OVER VAPOR RETARDER AND 4" DEPTH OF POROUS FILL UNLESS OTHERWISE INDICATED. REINFORCE SLAB WITH 4x4-W2.9xW2.9 WELDED WIRE REINFORCING PLACED 1 1/2" CLEAR BELOW TOP OF SLAB. MAINTAIN REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR SPACERS DURING CONCRETE PLACEMENT SPACED NOT MORE THAN 24" O/C.
- 02 6" CONCRETE SLAB-ON-GRADE OVER 4" DEPTH OF POROUS FILL UNLESS OTHERWISE INDICATED. REINFORCE SLAB WITH #4 AT 16" OC EACH WAY LOCATED 2" CLEAR BELOW TOP OF SLAB AND #5 AT 16" OC SHORT WAY LOCATED 1-1/2" CLEAR FROM BOTTOM OF SLAB STAGGERED WITH TOP BARS. MAINTAIN REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR SPACERS DURING CONCRETE PLACEMENT.
- 04 DOWELED CONTROL JOINT - LOCATION CRITICAL DUE TO HAIR PINS. LOCATION OF BRACED FRAME. SEE PEMB DRAWINGS FOR END BAY DESIGN. END BAYS HAVE NOT BEEN DESIGNED FOR FUTURE BUILDING EXPANSION.
- 06 PRE-MANUFACTURED GALVANIZED STEEL PLATFORM 10'x6' WITH STAIR AND HANDRAILS. TOP OF PLATFORM = +4'-0". PROVIDE VERTICAL LIFT GATE AT HYDRAULIC LIFT.
- 09 PRE-MANUFACTURED CANOPY COLUMN. REFERENCE ARCHITECTURAL DRAWINGS FOR CANOPY.
- 10 THICKENED SLAB AT PERIMETER. REFERENCE SECTIONS.
- G1A ALTERNATE G1A: REFERENCE ARCHITECTURAL DRAWINGS FOR BID/ALTERNATE DESCRIPTION.
- G2 ALTERNATE G1A: REFERENCE ARCHITECTURAL DRAWINGS FOR BID/ALTERNATE DESCRIPTION.
- X1 EXISTING WALL.
- X2 EXISTING SLAB.

KEY PLAN



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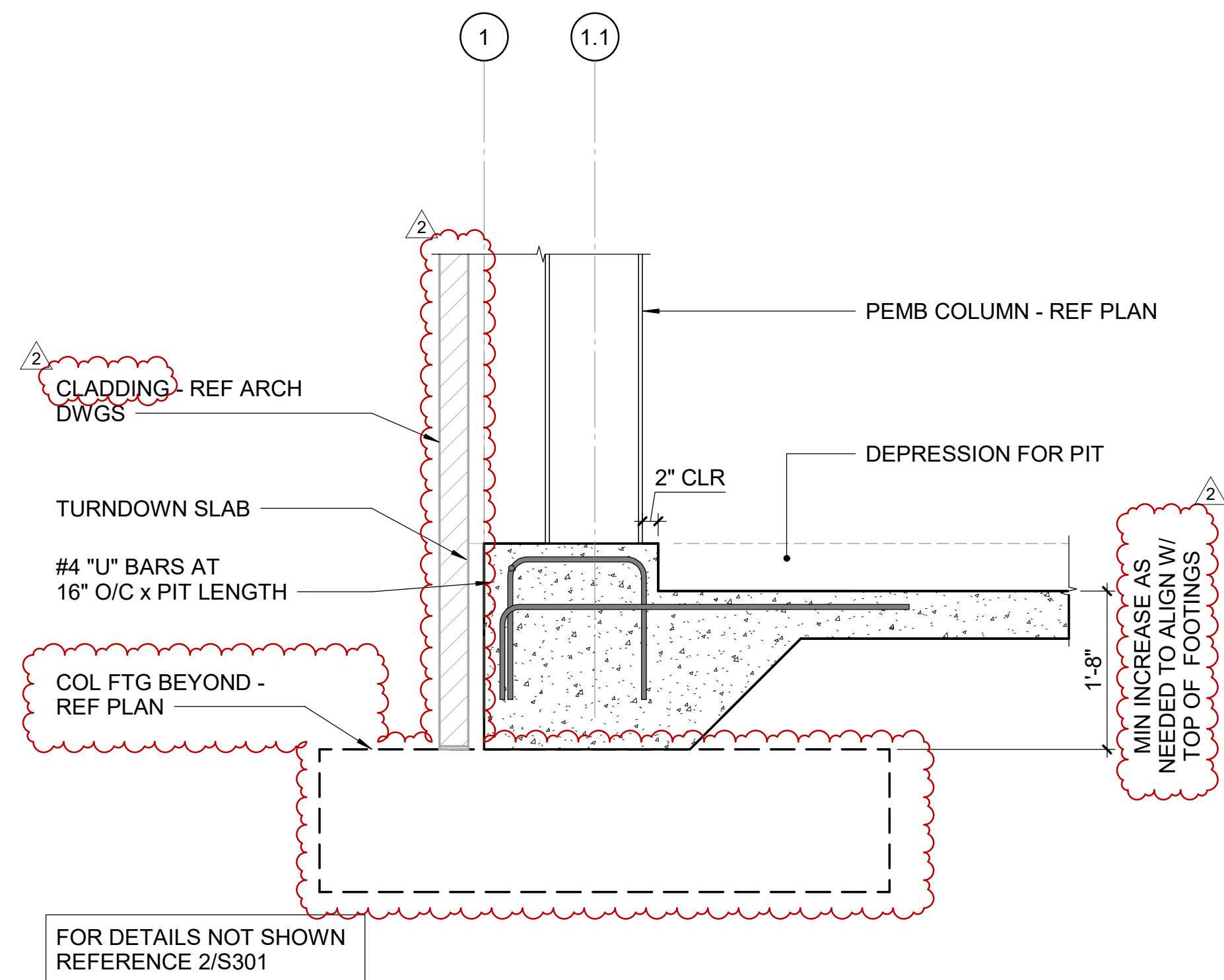
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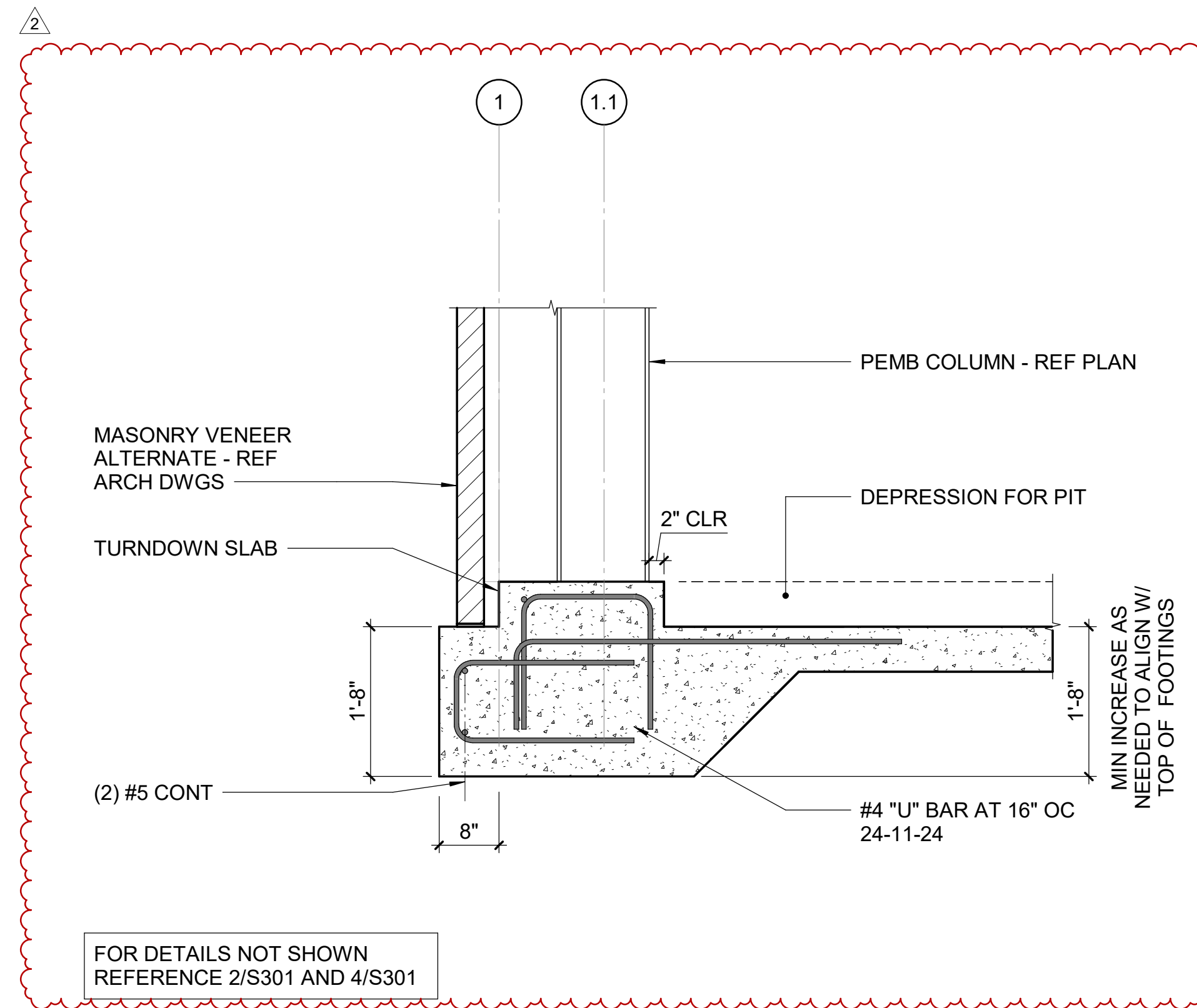
SLAB PLAN

DATE
09-06-24

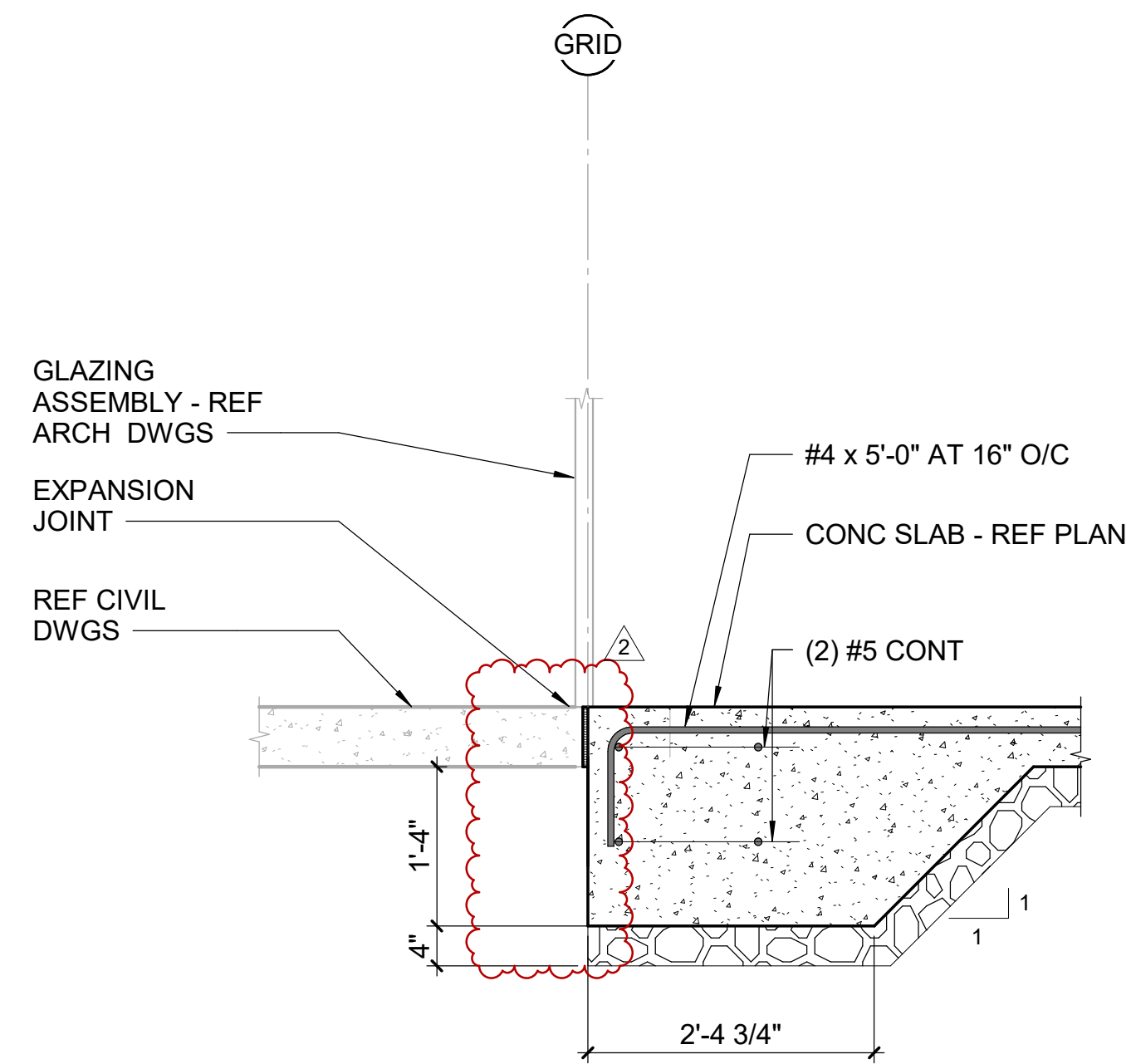
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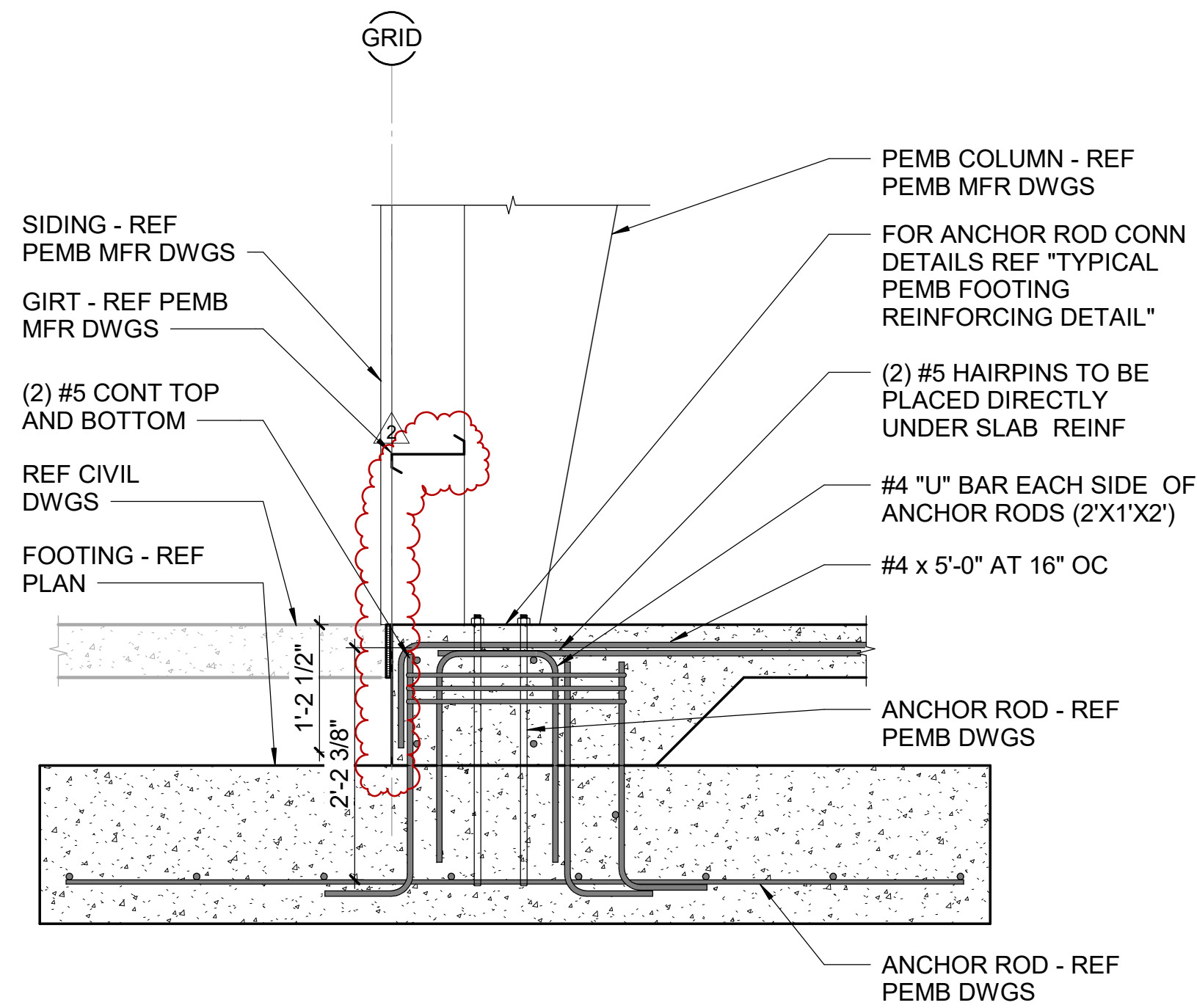
4 SECTION
3/4" = 1'-0"



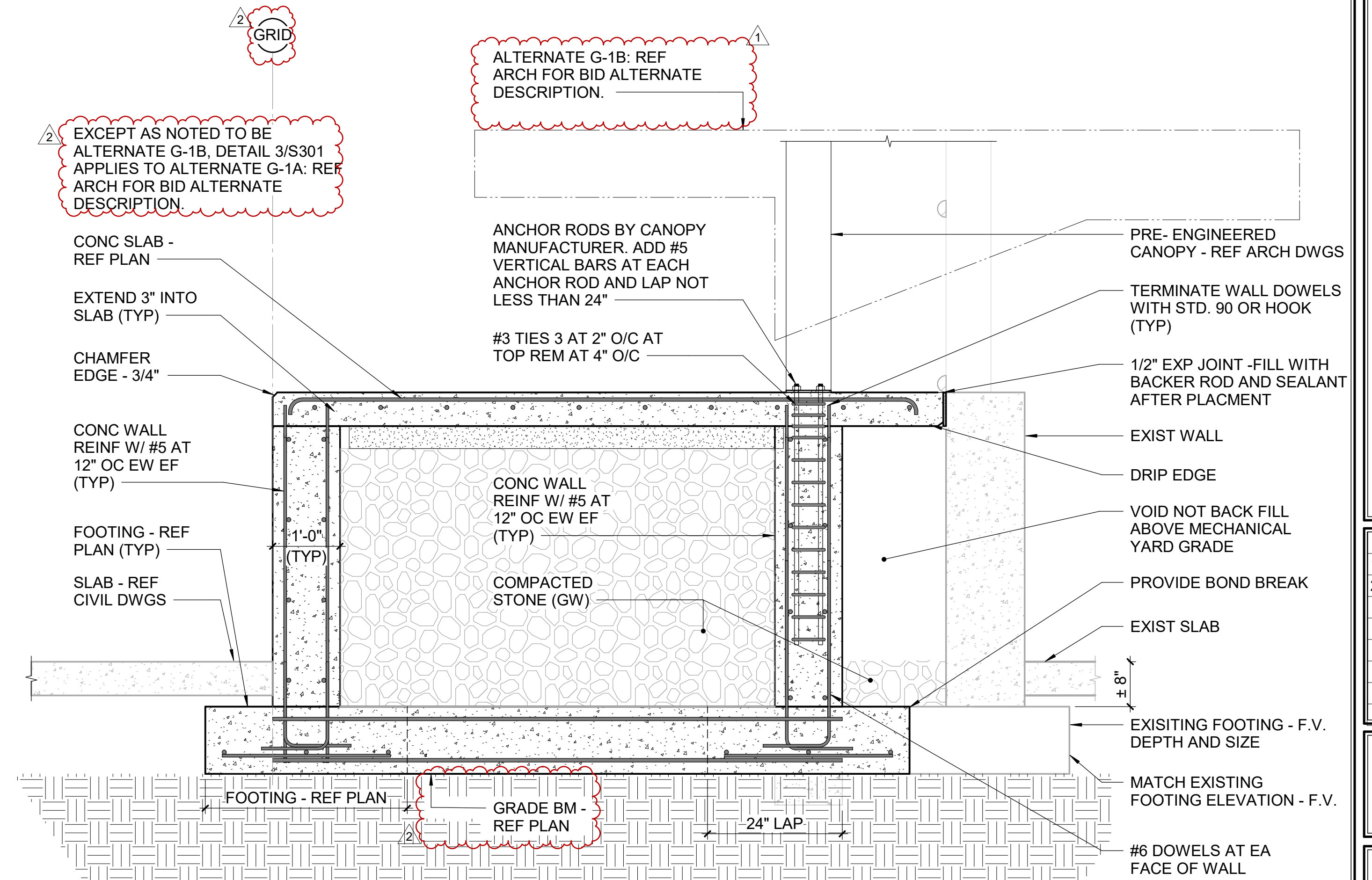
5 SECTION ALTERNATE G-3 BRICK
3/4" = 1'-0"



1 SECTION
3/4" = 1'-0"

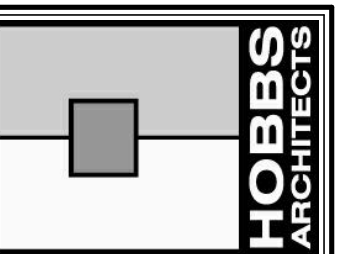


2 SECTION
3/4" = 1'-0"



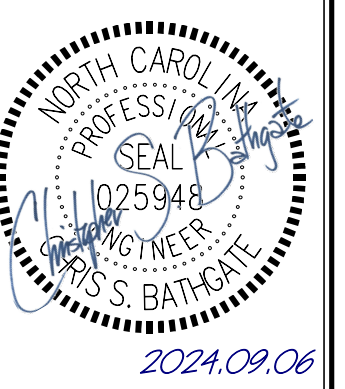
3 SECTION
3/4" = 1'-0"

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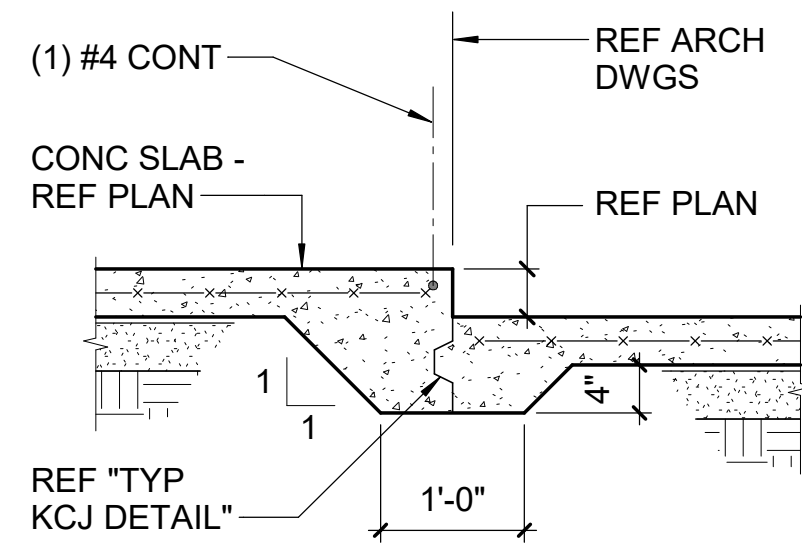
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SECTIONS

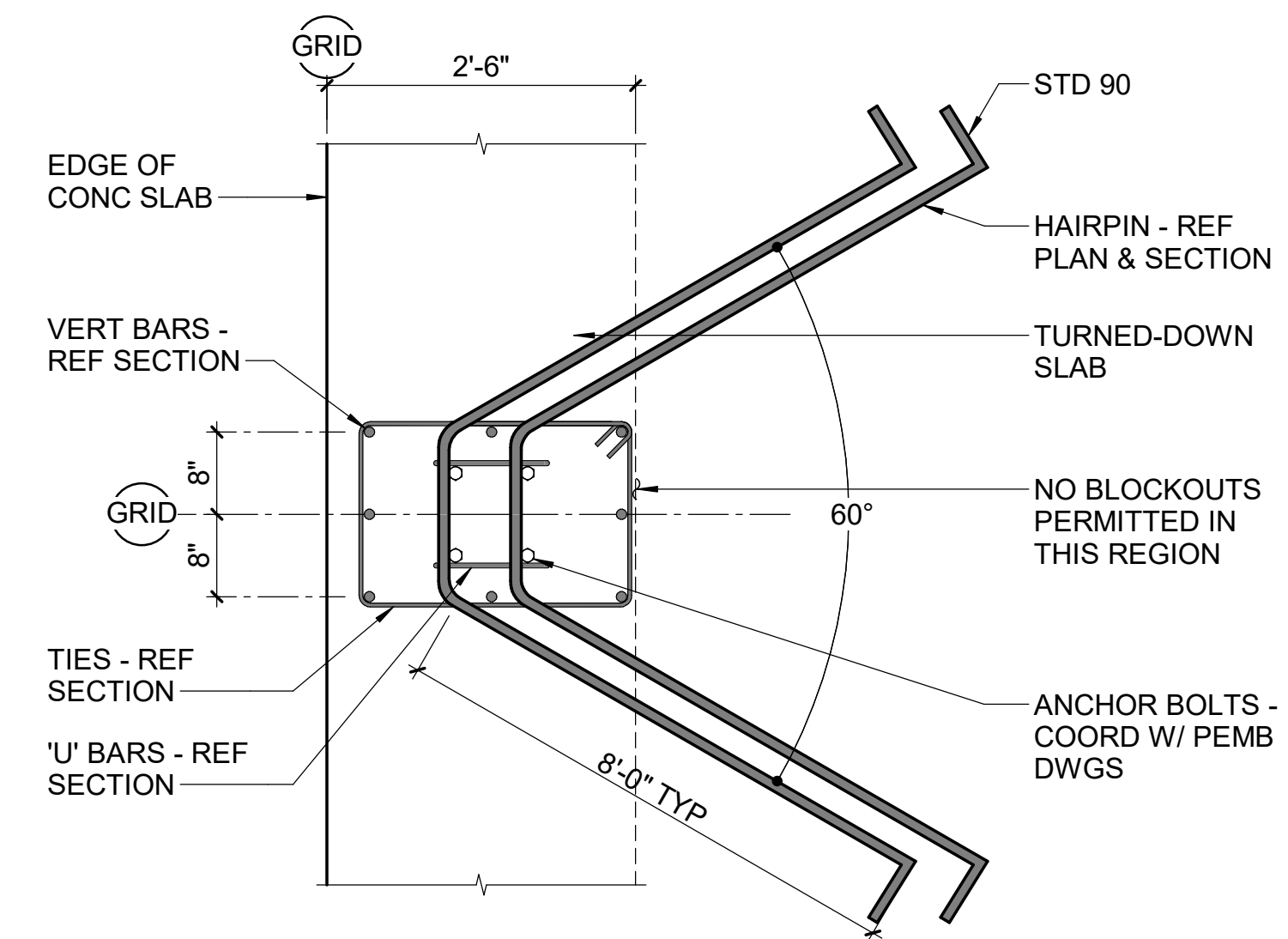
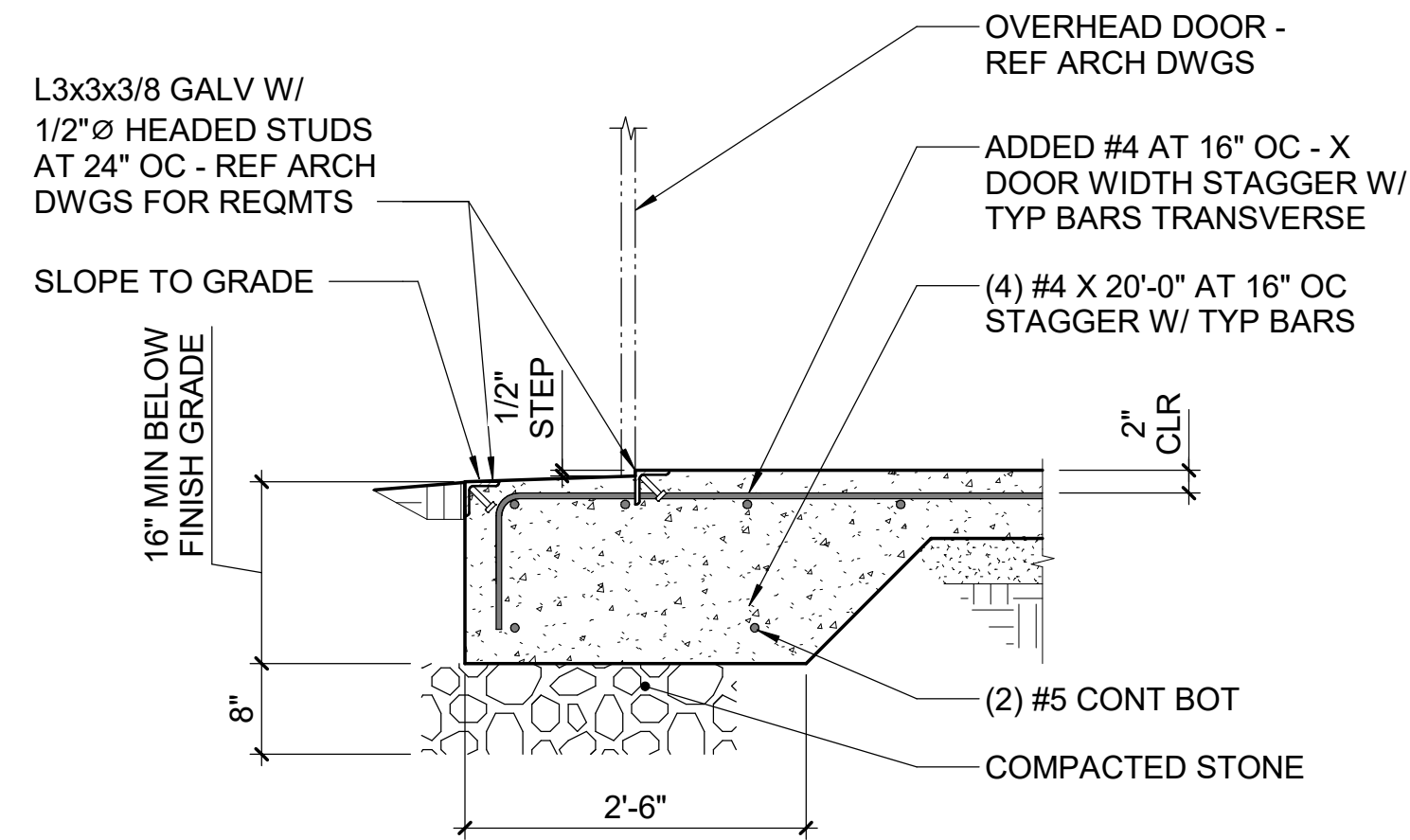
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S301



GREATER THAN 4"

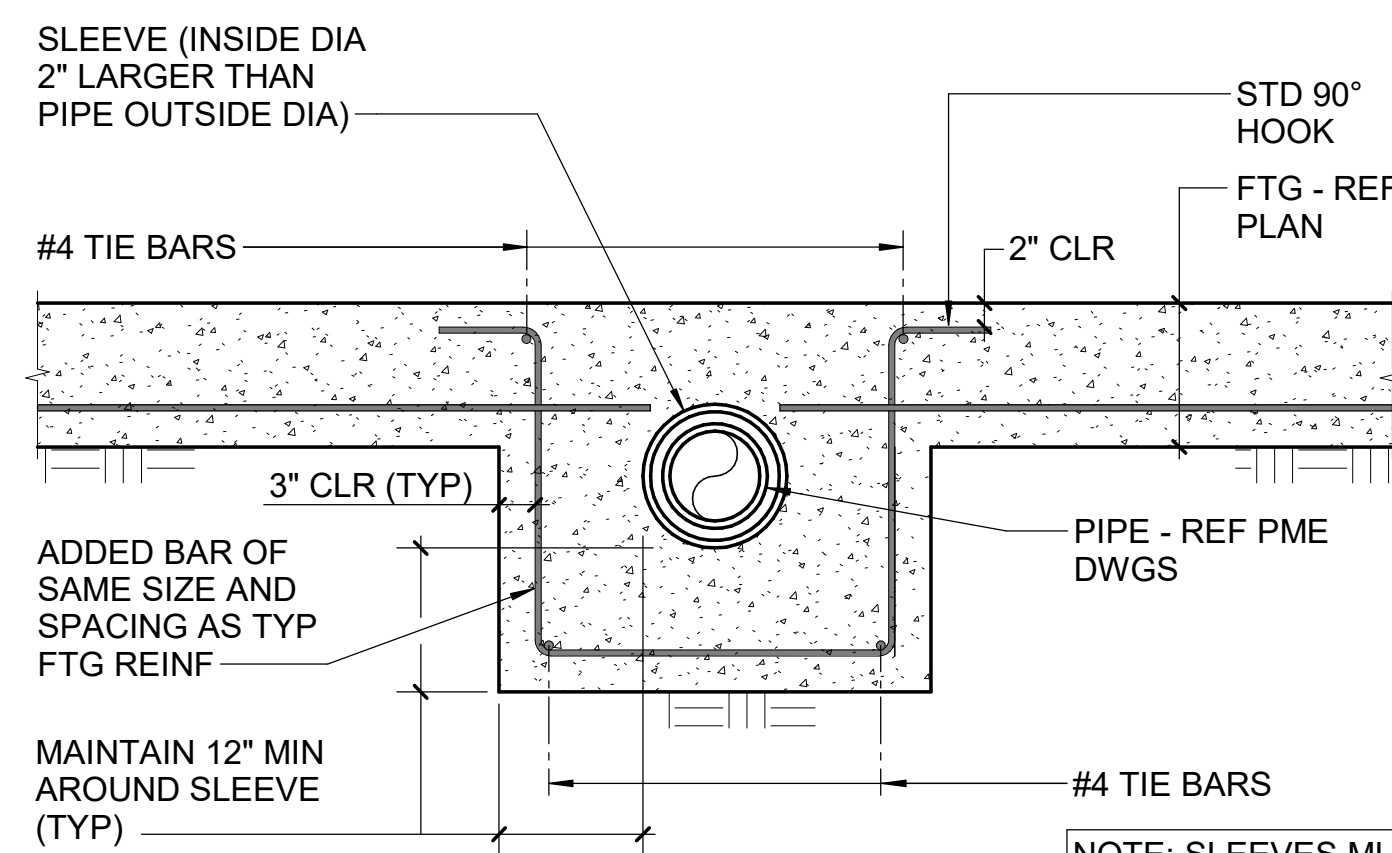
DENOTED THUS: ON PLAN



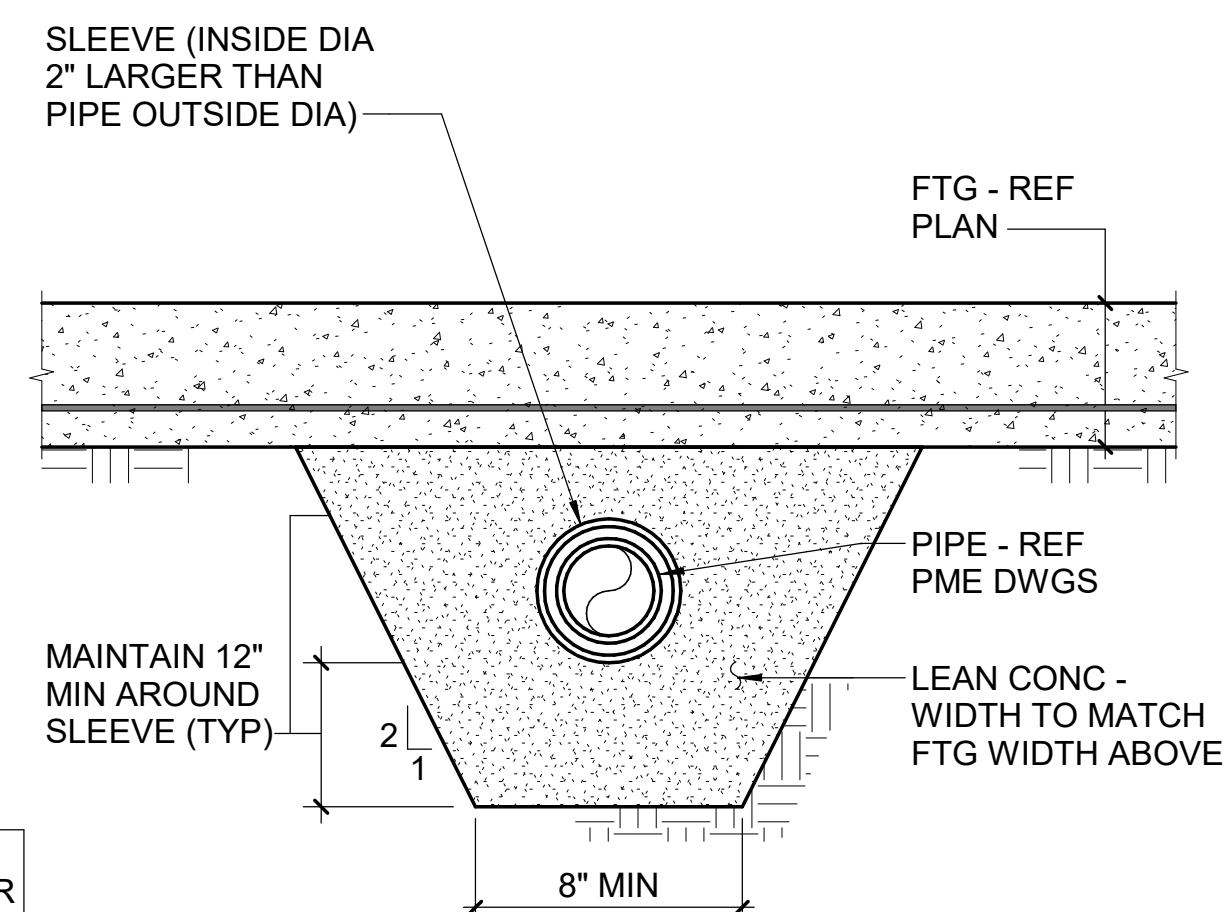
7 TYPICAL DEPRESSED SLAB DETAILS
3/4" = 1'-0"

6 TYPICAL FOUNDATION AT OVERHEAD DOORS DETAIL
NTS

8 TYPICAL PEMB FOOTING REINFORCING DETAIL
3/4" = 1'-0"

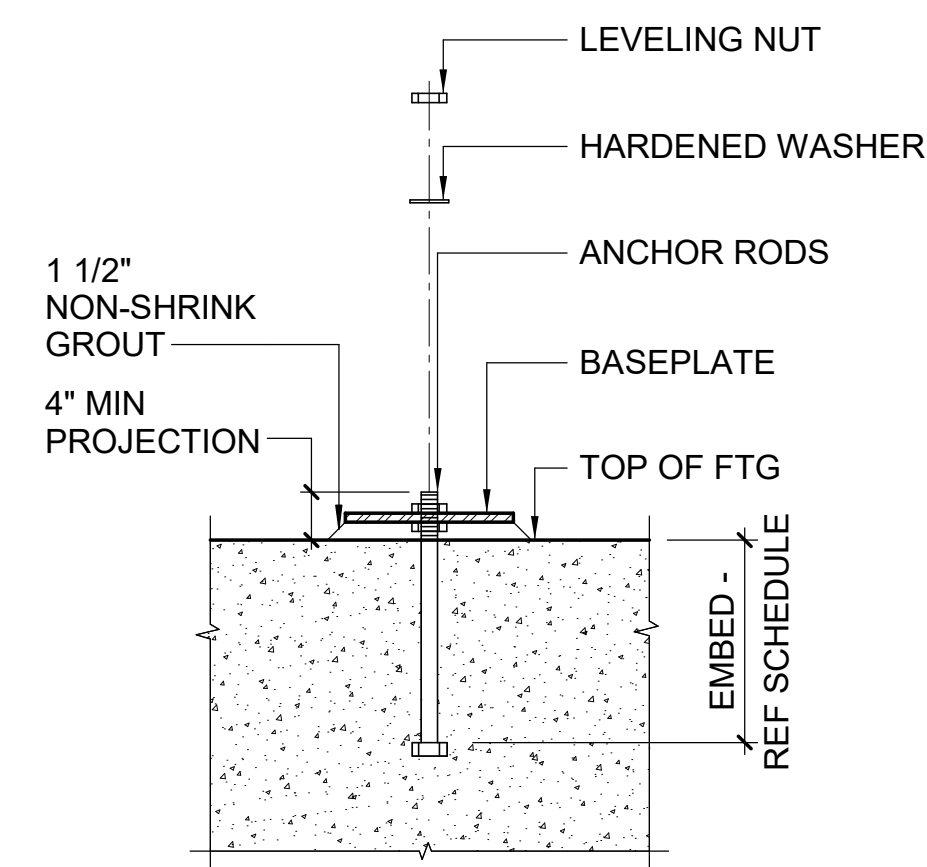


THROUGH FOOTING



BELOW FOOTING

NOTE: SLEEVES MUST NOT BE LOCATED IN OR UNDER COL FTGS.



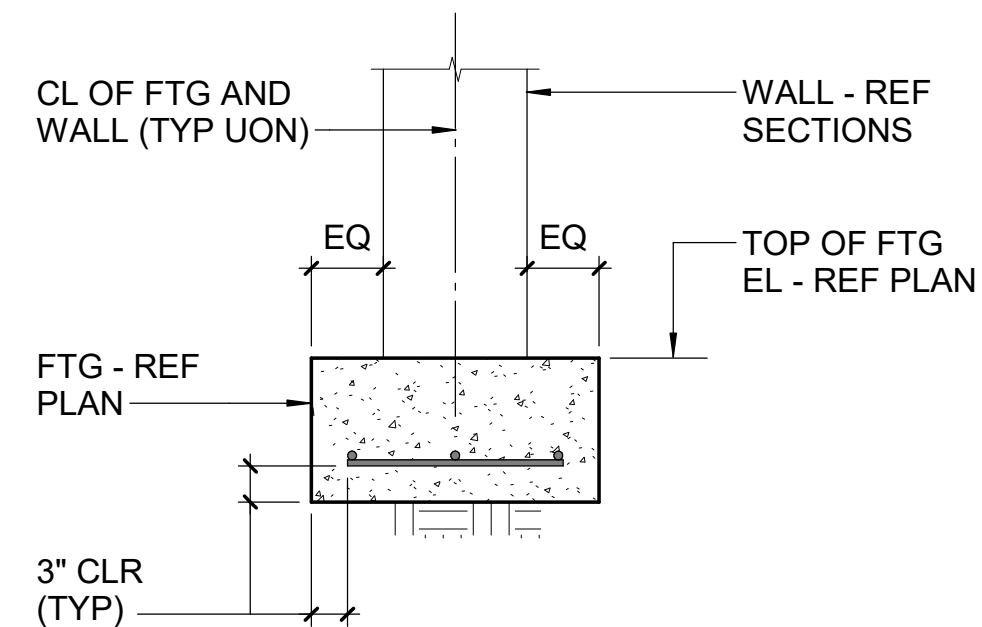
NOTE: FTG REINF NOT SHOWN.

ANCHOR ROD EMBEDMENT SCHEDULE	
ANCHOR ROD DIAMETER	EMBEDMENT
3/4" Ø	12"
1" Ø	12"

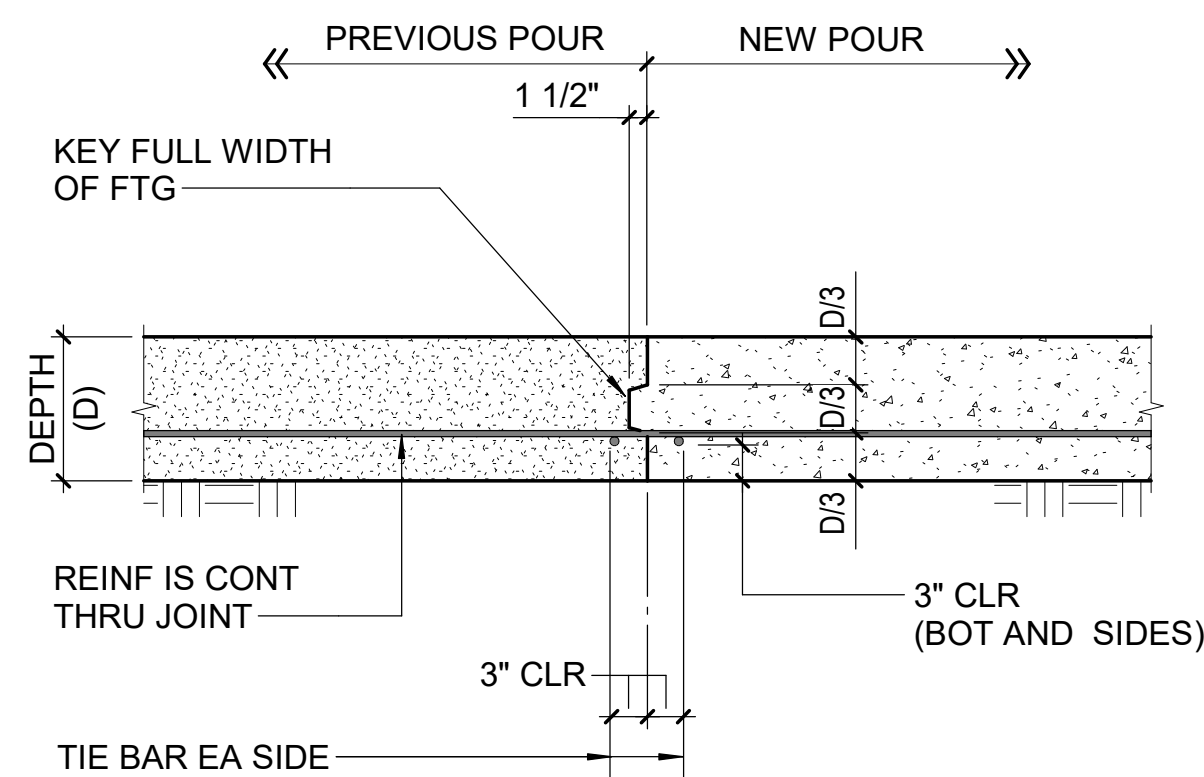
NOTE: ALL ANCHOR RODS SHALL BE ASTM F1554, GRADE 55 OR BETTER WITH HEAVY HEX NUTS TACK WELDED TO BOTTOM OF ANCHOR RODS.

5 TYPICAL PIPE SLEEVE AT WALL FOOTING DETAILS
NTS

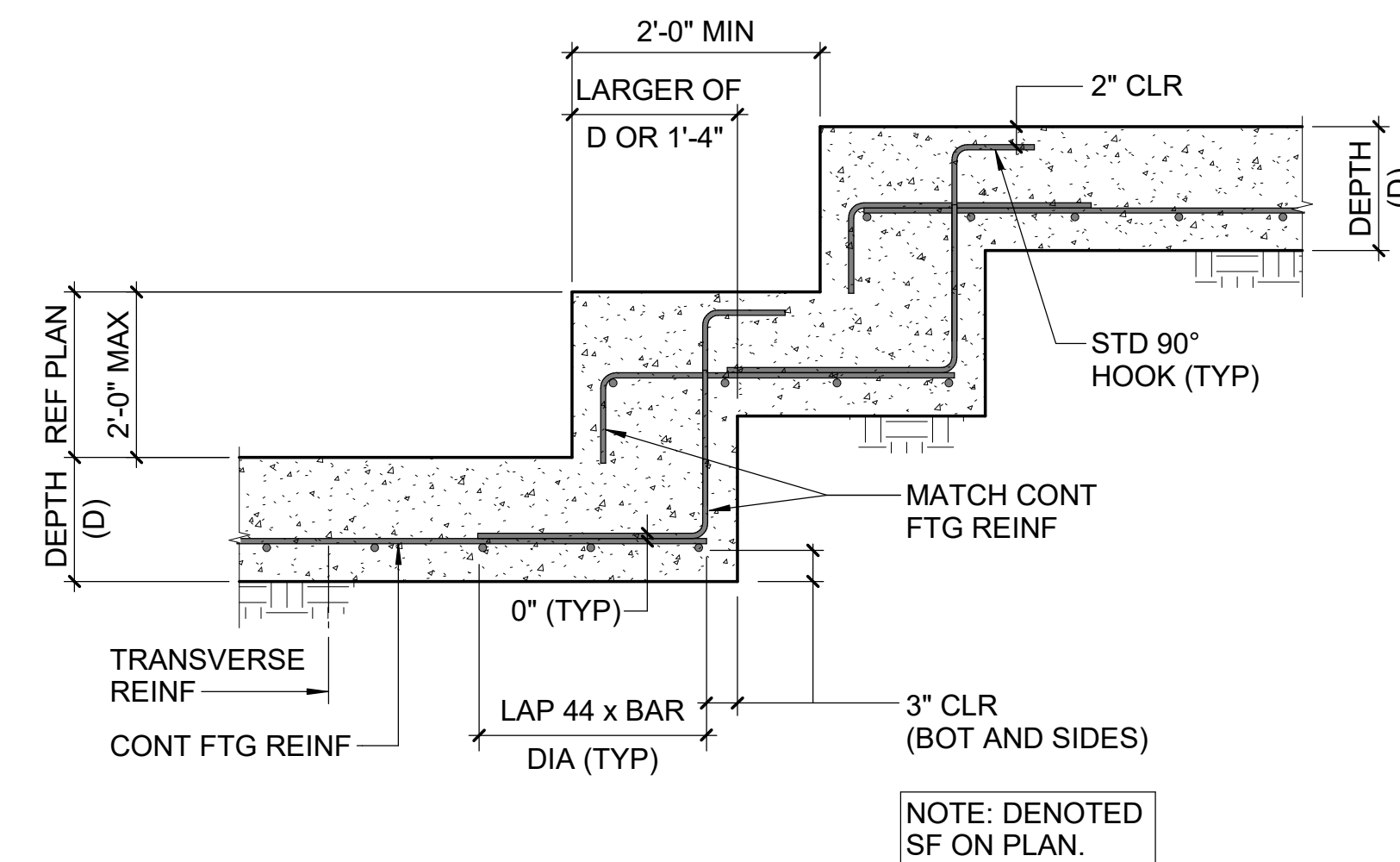
3 TYPICAL ANCHOR ROD EMBEDMENT DETAIL
NTS



1 TYPICAL WALL FOOTING DETAIL
3/4" = 1'-0"

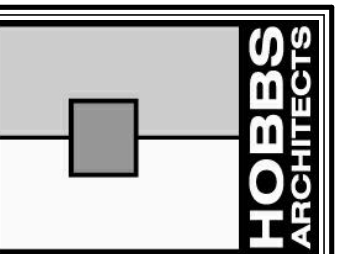


4 TYPICAL WALL FOOTING CONSTRUCTION JOINT DETAIL
NTS



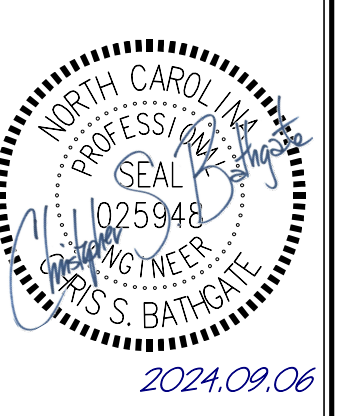
NOTE: DENOTED SF ON PLAN.

2 TYPICAL STEPPED WALL FOOTING DETAIL
3/4" = 1'-0"



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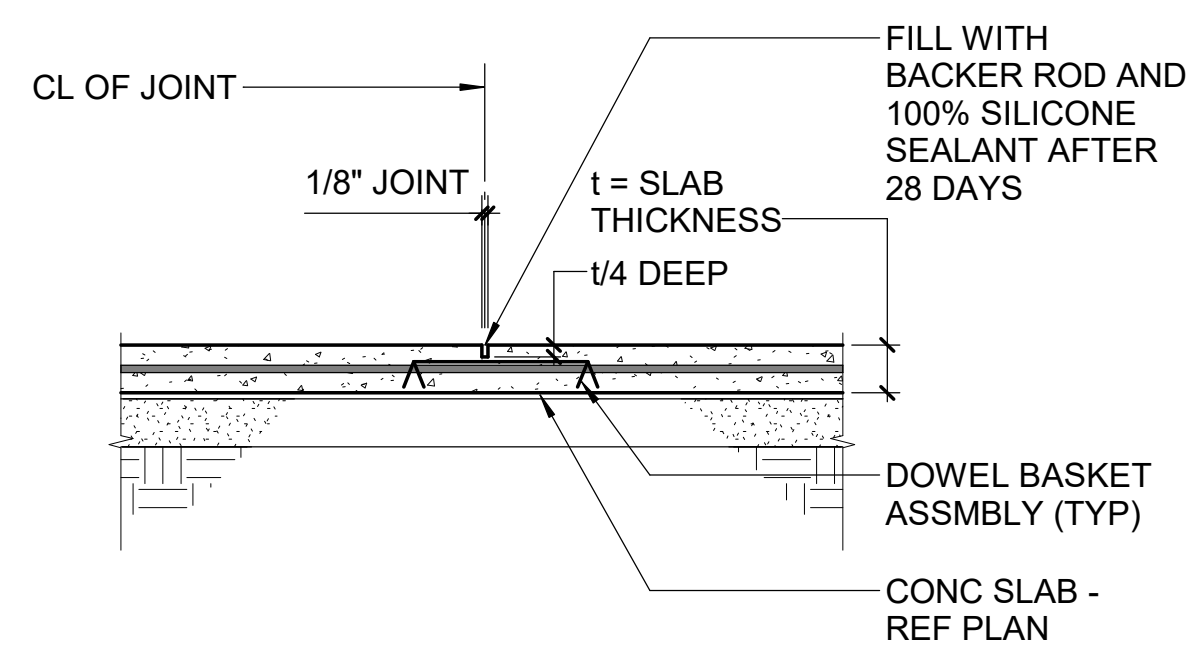
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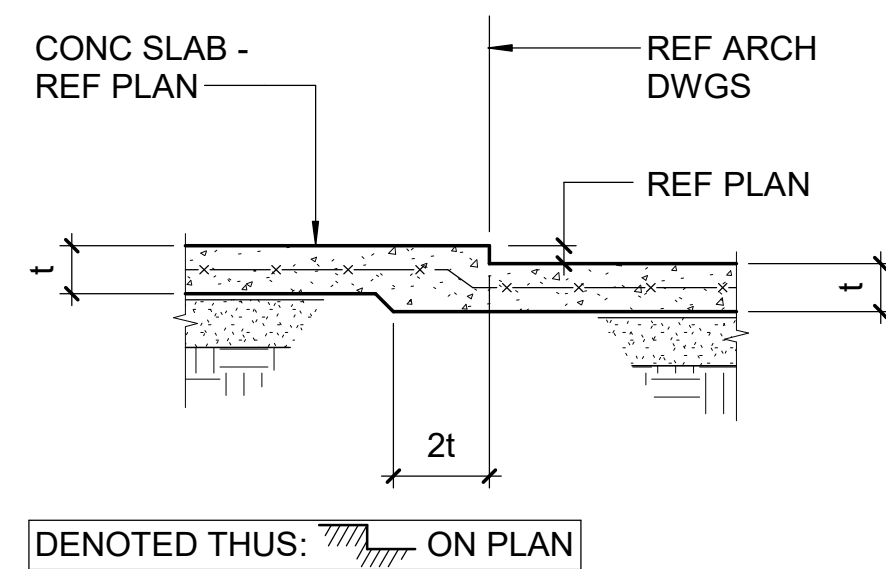
TYPICAL DETAILS

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S501

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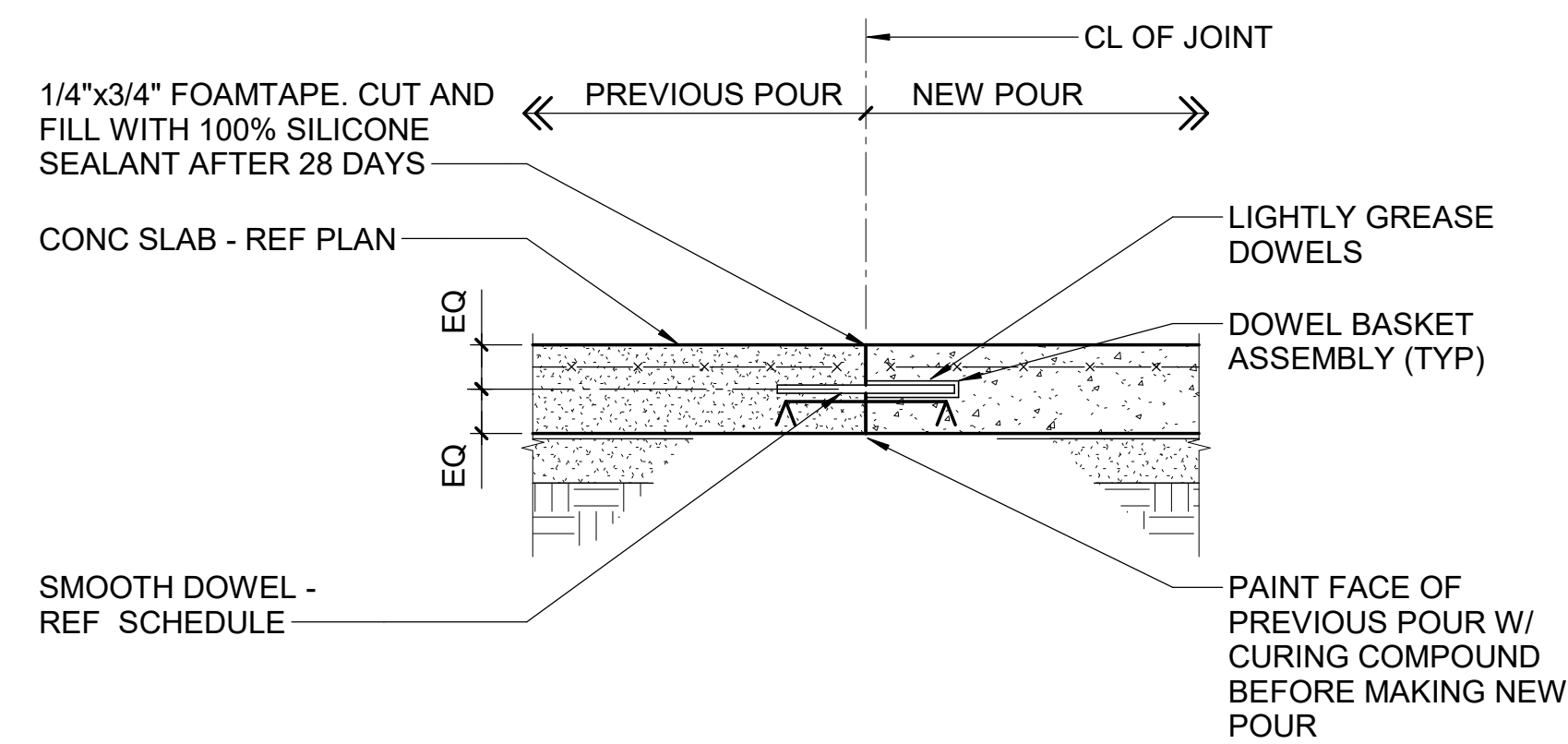


1 TYPICAL SAWED JOINT DETAIL
3/4" = 1'-0"

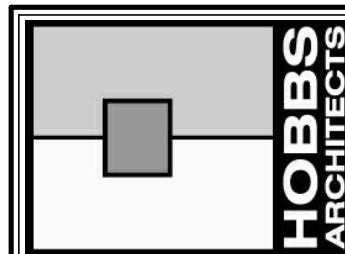


3 TYPICAL DEPRESSED SLAB DETAIL
3/4" = 1'-0"

SMOOTH DOWEL SCHEDULE		
SLAB DEPTH	SIZE	SPACING
6"	3/4"Ø x 14" LONG	12"OC

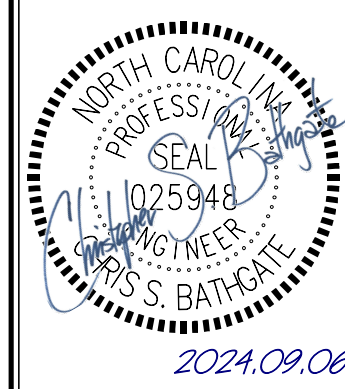


2 TYPICAL DOWELED CONSTRUCTION JOINT DETAIL
3/4" = 1'-0"



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TYPICAL DETAILS

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S502